EBA Engineering Consultants Ltd.

Civil, Geotechnical and Materials Engineers



GRANULAR BORROW INVESTIGATION SNOWDRIFT AIRPORT SNOWDRIFT, N.W.T.



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April, 1992

0701-10609



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Submitted to:

GOVERNMENT OF THE NORTHWEST TERRITORIES DEPARTMENT OF TRANSPORTATION YELLOWKNIFE, N.W.T.

Prepared by:

EBA ENGINEERING CONSULTANTS LTD. YELLOWKNIFE, N.W.T.

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1.0 INTRODUCTION

During March, 1992, EBA Engineering Consultants Ltd. (EBA) undertook a granular investigation for the proposed airstrip and access road in the community of Snowdrift, N.W.T. The investigation was at the request of Mr. Mikko Nyyssonen, P.Eng., Geotechnical Engineer, Department of Transportation, Government of the Northwest Territories. The work was carried out under Work Order No. 92002; Contract Agreement No. 295248.

The purpose of the investigation was to identify sufficient quantities of crushable granular material (14,000 cu.m.), embankment fill (30,000 cu.m.) and blend sand (5,000 cu.m.) for the construction of the airstrip and access road, as well as to perform general terrain mapping along the winter haul route from Snowdrift to borrow sources along the Snowdrift River.

2.0 REVIEW OF EXISTING INFORMATION

A preliminary review of existing information was conducted prior to commencement of the field program. Material reviewed consisted of the following (supplied by GNWT Transportation):

- Aerial photos (Roll # A26547, photos #43 to 68, scale 1:20,000)
- Report of Transport Canada gravel search 1983
- Report of GNWT DPW investigation of Site 6 1990
- Report of GNWT Transportation investigation of Site 5
- Report of GNWT Transportation investigation of potential quarry rock sources - 1991
- Topographic map # 75 L/7, scale 1:50,000.
- Specifications for airport surface material

3.0 FIELD WORK

The investigation was undertaken with the understanding that the only available equipment for excavation of test pits would be a Caterpillar D-6 bulldozer operated by the Lutzel k'e Economic Development Corporation in Snowdrift. EBA's field representative was Mr. T.L. Jordan, Senior Technologist. The following is a description of field activities.

3.1 Site 5

Work commenced on March 11,1992. Test pits were attempted at three locations at Site 5, identified as EBA-1 to EBA-3, on Figure 1, Appendix A. The material encountered was frozen coarse



gravel. The bulldozer was unable to excavate deeper than about 0.1 metre at any of the three locations. Therefore Site 5 was abandoned and the bulldozer was moved to Site 4.

3.2 Site 4

On March 12, test pits were attempted at five locations at Site 4, identified as EBA-4 to EBA-8, on Figure 1, Appendix A. Findings were as follows:

EBA-4: The bulldozer was unable to penetrate the frozen organic material on the surface. No test pit was possible at this location.

EBA-5: At least one metre depth of silty sand, some cobble and gravel, was found at this site. The bulldozer was unable to excavate deeper than 1 metre in the frozen ground.

EBA-6: The bulldozer was able to dig only to a depth of 0.8 metre at this location. Material encountered was similar to that at EBA-5.

EBA-7: Material just below the organic layer was a well-graded clean sand. After excavating to a depth of about 0.1 metre it was discovered that one section of cutting edge had been lost. The investigation in Site 4 was abandoned and field activities were delayed pending delivery of a new cutting edge.

EBA-8: A test pit was attempted at this location prior to EBA-7. The bulldozer was unable to penetrate the frozen organic surface material.

Samples were obtained from EBA-5 and EBA-6 and transported to EBA's Yellowknife laboratory. Washed sieve analyses were performed on these two samples. Results are presented in Appendix B.

3.3 Site 6

The investigation was resumed on March 17, 1992. During March 17th and 18th, the bulldozer travelled up the trail adjacent to the Snowdrift River to Sites 6 and 7. On March 19th, three test pits were excavated at Site 6. These pits are identified as EBA-9 to EBA-11, on Figure 2, Appendix A. A composite sample of the granular material was retrieved from each test pit and sent to EBA's Yellowknife laboratory. Laboratory testing consisted of washed sieve analyses of pitrun, laboratory crushing followed by washed sieve analyses, Los Angeles abrasion tests and fractured face counts. Logs of these test pits and results of laboratory testing are included in Appendix B.



3.4 <u>Site 7</u>

On March 20, 1992, three test pits were excavated at Site 7. These are identified as EBA-12 to EBA-14, on Figure 3, Appendix A. Sampling and testing was the same as for Site 6. Logs of these test pits and results of laboratory testing are included in Appendix B.

4.0 SITE DESCRIPTIONS

4.1 Site 4

Site 4 comprises an area of approximately 300 by 1400 metres to the north of the Snowdrift Airport runway, between the runway and the lower Snowdrift River, which runs from Stark Lake into Great Slave Lake. The area is a flood plain, heavily treed with black spruce. It is bounded on the southeast by the runway and descends in a series of shallow terraces down to muskeg areas along the river on the northwest. The area to the west of Site 4 has been used as a source of limited amounts of borrow material in the past.

4.2 <u>Site 5</u>

Site 5 lies along a narrow ridge extending west from the base of an exposed bedrock knob, comprising an area of approximately 150 by 650 metres. It is southeast of the runway and separated from it by a small pond and muskeg area. The area has been investigated by GNWT Transportation and found to contain sand and gravel to a depth of up to two metres, underlain by permafrost.

4.3 <u>Site 6</u>

Site 6 is located along the west shore of the upper Snowdrift River approximately four kilometres upstream from the entrance to Stark Lake. The site area is a river terrace about three to four metres above the present river level, and about 7000 square metres in area. It is lightly treed with jackpine. Site 6 was found to contain excellent quality gravel with some sand and cobble to a depth of at least 2.5 metres. It is surrounded to the south, east and west by higher terraces composing part of a much larger glacial wash deposit containing gravelly sand to a similar depth.



4.4 Site 7

Site 7 is a gravel bar slightly higher than the present river level. The total area of the gravel bar is in excess of 100,000 square metres. Except for a small island within the downstream end of the bar, it has virtually no vegetation. The extended area researched upstream along the left bank of the river is fairly heavily treed with jackpine. Previous investigations had found cobbly gravel to a depth of at least 1.5 metres in the gravel bar itself. The island and the left bank were found to contain one metre depth of excellent quality gravel with slightly more cobble than at Site 6.

5.0 DISCUSSION

5.1 Overland Route

An area map showing the route and all four borrow sites is presented in Figure 4, Appendix A. The alignment of the route shown on this map was taken from a survey traverse performed by Mr. Wayne Nesbitt of GNWT Department of Public Works, for the section from Station 2+000 to 8+000, and from aerial photos for the remainder of the route.

Appendix C of this report contains a narrative description of the terrain encountered along the route to Sites 6 and 7, including information about soils and vegetation. Locations of photographs are noted. The photographs are included in Appendix D.

5.2 Sites 4 and 5

Due to the difficulties encountered in attempting to excavate test pits in Sites 4 and 5, we were unable to prove quantities of material in these areas. It is recommended that a backhoe be hired during the summer months to investigate these sites.

From the limited information acquired at Site 4, it appears probable that sufficient blend material does exist. Results of gradation tests on samples from locations EBA-5 and EBA-6 indicate that the material is suitable for blending with crushed rock to produce a suitable surface material. However both test pits contained about 20% of gravel, to a maximum size of 60 mm.

5.3 <u>Site 6</u>

Previous quantity estimates for Site 6 (GNWT-1990) have assumed a gravel thickness of 1.5 metres. Based on results of the present investigation it is reasonable to assume a depth of 2.5 metres



over the area previously proven. Also, stripping losses can be reduced by limiting stripping to the organic peat and tree roots, leaving in place as much as possible of the silt which overlies the gravel. This silt will mix with the gravel during handling and processing, bringing up the quantity of fines in the product.

Assuming 7000 square metres at a depth of 2.5 metres results in an estimated volume of 17,500 cubic metres minus 700 cubic metres organic overburden, or 16,800 cubic metres proven. The test pit at EBA-11 indicates that the material in the upper terrace surrounding Site 6 contains a greater percentage of gravel sizes than was previously assumed. Therefore it is probable that Site 6 and the region around it contain considerably more material that can be made suitable by crushing and/or screening.

5.4 <u>Site 7</u>

Excavation of the main gravel bar at Site 7 proved to be impossible with the bulldozer at this time of year. Because of this, test pits were attempted on the island at the downstream end of the bar and on the left shore upstream of the gravel bar, with a view to extending knowledge of the deposit to a greater area.

EBA-12, on the island, indicated at least one metre thickness of crushable granular material underlain by frozen saturated gravel of undetermined depth. Results of test pitting at EBA-13 and EBA-14 indicate a thickness of about one metre of granular material overlain by from 0.2 to 1.2 metres of silt and silty sand.

Previous reports have not given a quantity estimate for Site 7, but have assumed a depth of 1.5 metres of recoverable material. Assuming a usable area of 20,000 square metres, this results in a proven volume of approximately 30,000 cubic metres, in excess of project requirements. The present investigation indicates that probably a much larger quantity of crushable material is available, considering the extended area investigated.

Transport Canada's 1983 report indicated that material would be hauled for the most part over ice roads on the river and lake. From this point of view they concluded that there was little difference between Sites 6 and 7, aside from the slightly greater distance to Site 7 and slightly more cobble-sized material at Site 7, and they recommended that Site 7 be used as the borrow source.

We understand that current plans are for an overland route. Access from this route to the source at Site 7 is longer and more



difficult and would require more construction than the corresponding access to Site 6. Also, the shallower depth of material at Site 7 would result in the disturbance of a greater area (up to 10,000 square metres) in order to recover the required quantity.

We understand that Site 7 is a popular berry picking and fishing site for the community of Snowdrift. For these reasons it would be prudent to consider community attitudes and plans prior to making a final decision on choice of sites. Either site contains sufficient material of superior hardness which can be processed to provide surfacing material for the runway and access road.

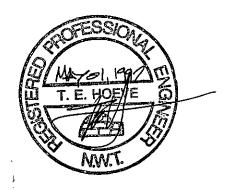
6.0 CLOSURE

We trust that you will find this report sufficient to your present requirements. If you have any questions or comments, please contact the undersigned.

Respectfully submitted, EBA Engineering Consultants Ltd.

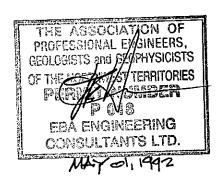
Terrence L. Jordan Senior Technologist

Reviewed by



T.E. Hoeve, P. Eng. Senior Project Engineer

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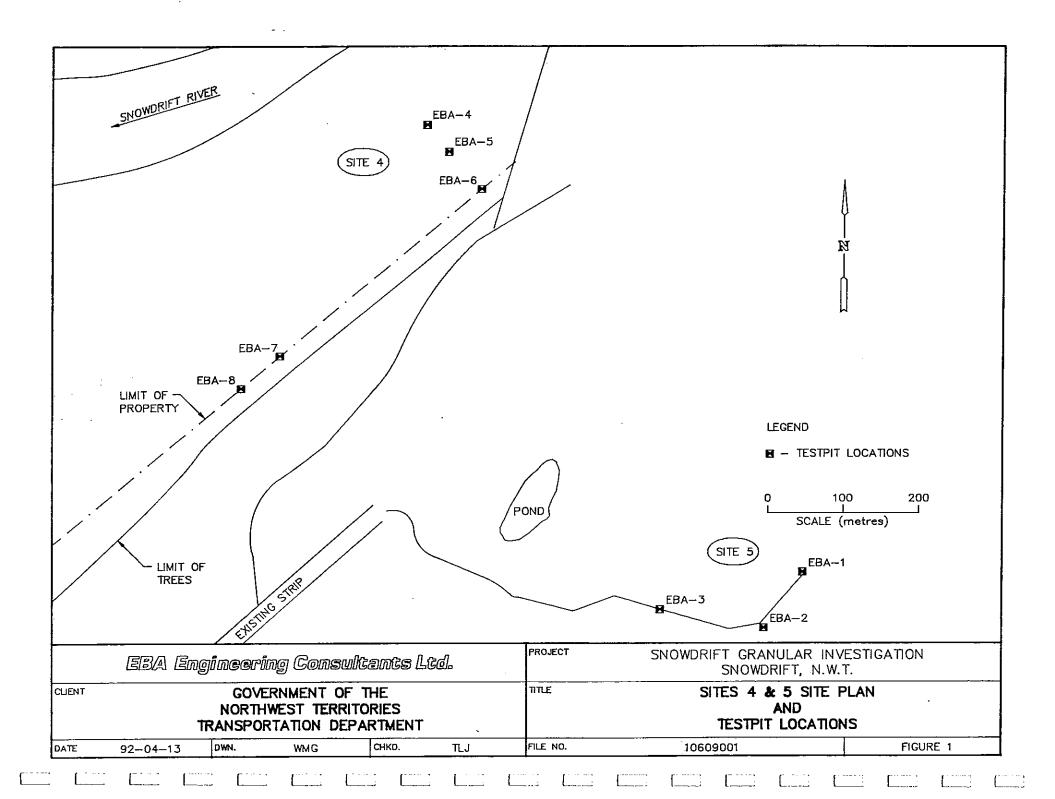


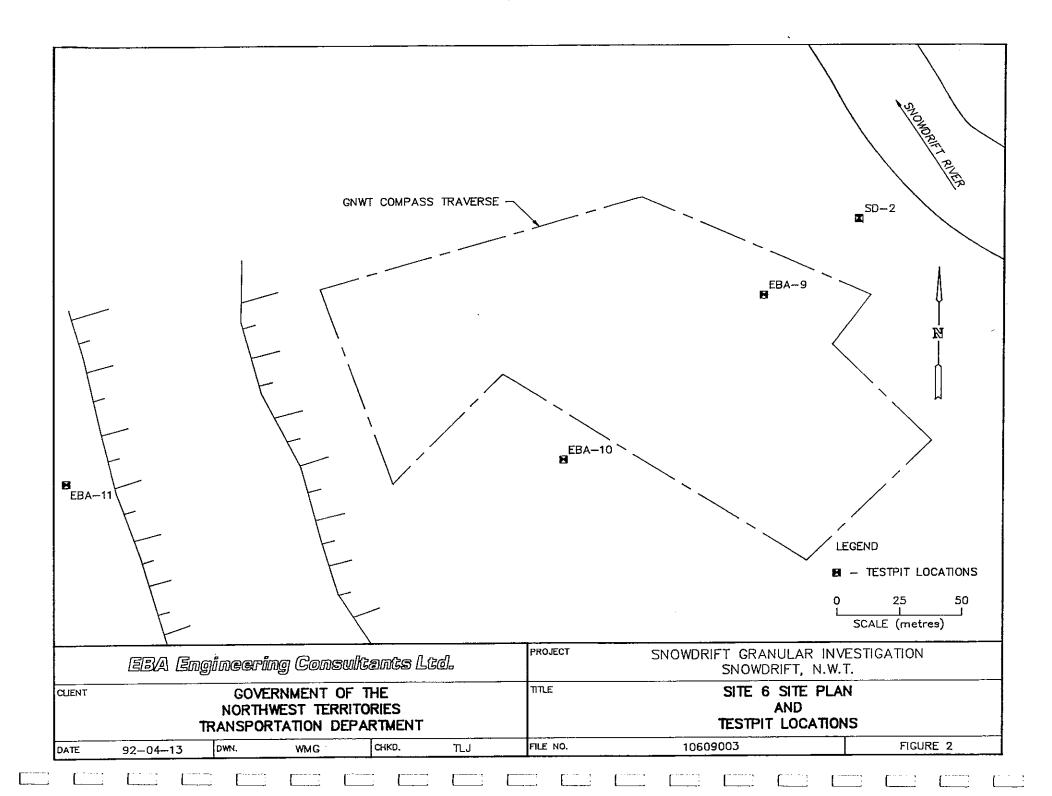


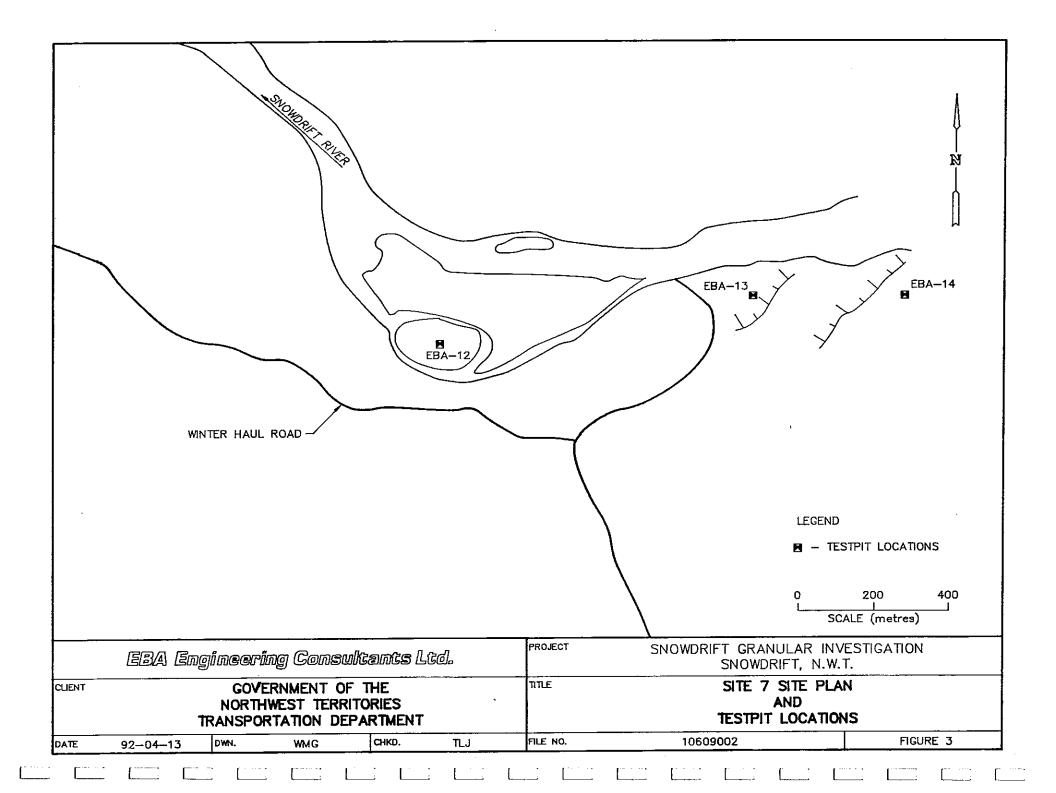
APPENDIX A

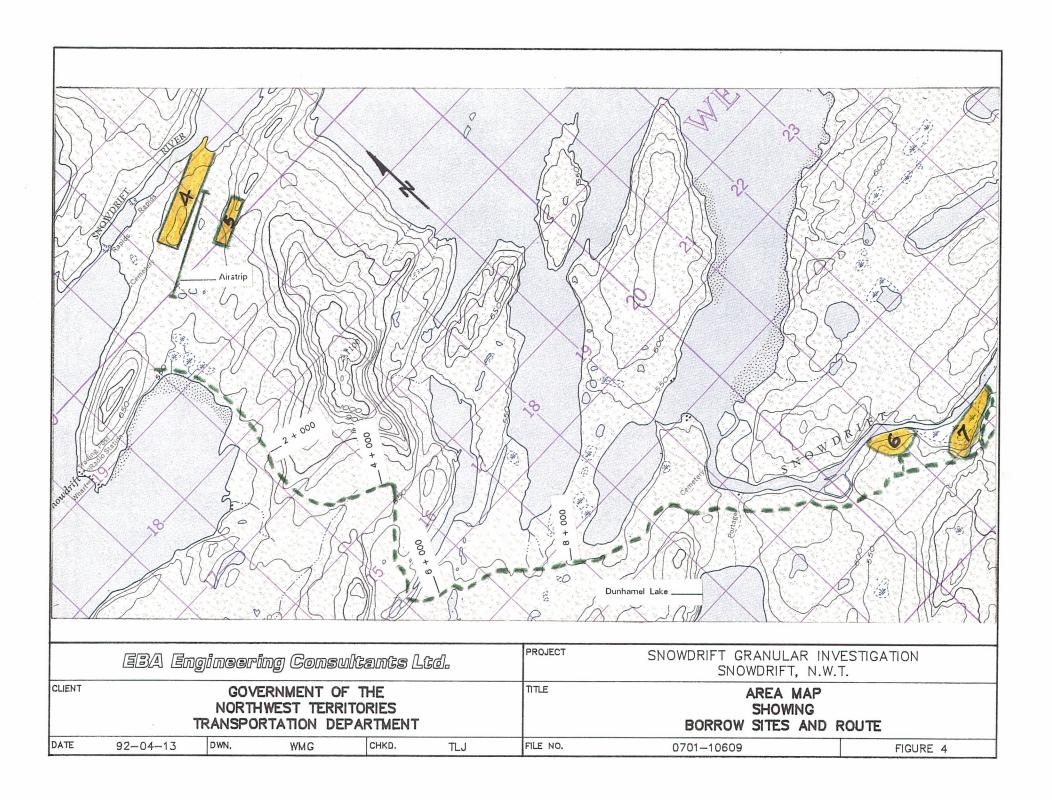
FIGURES











APPENDIX B

TEST PIT LOGS
AND
RESULTS OF LABORATORY TESTING



TERMS USED ON BOREHOLE LOGS

TERMS DESCRIBING CONSISTENCY OR CONDITION

COARSE GRAINED SOILS (major portion retained on 0.075mm sieve): includes (1) clean gravels and sands, and (2) silty or clayey gravels and sands. Condition is rated according to relative density, as inferred from laboratory or in situ tests.

DESCRIPTIVE TERM	RELATIVE DENSITY	N (blows per 0.3m)
Very Loose	0 to 20%	0 to 4
Loose	20 to 40%	4 to 10
Compact	40 to 75%	10 to 30
Dense	75 to 90%	30 to 50
Very Dense	90 to 100%	greater than 50

The number of blows, N, on a 51mm O.D. split spoon sampler of a 63.5kg weight falling 0.76m, required to drive the sampler a distance of 0.3m from 0.15m to 0.45m.

FINE GRAINED SOILS (major portion passing 0.075mm sieve): includes (1) inorganic and organic silts and clays, (2) gravelly, sandy, or silty clays, and (3) clayey silts. Consistency is rated according to shearing strength, as estimated from laboratory or in situ tests.

DESCRIPTIVE TERM	UNCONFINED COMPRESSIVE STRENGTH (kPa)
Very Soft	Less Than 25
Soft	25 to 50
Firm	50 to 100
Stiff	100 to 200
Very Stiff	200 to 400
Hard	Greater Than 400

NOTE: Slickensided and fissured clays may have lower unconfined compressive strengths than shown above, because of planes of weakness or cracks in the soil,

GENERAL DESCRIPTIVE TERMS

Slickensided - having inclined planes of weakness that are slick and glossy in appearance. Fissured - containing shrinkage cracks, frequently filled with fine sand or silt; usually more or Laminated - composed of thin layers of varying colour and texture. Interbedded - composed of alternate layers of different soil types. - containing appreciable quantities of calcium carbonate. Calcareous

- having wide range in grain sizes and substantial amounts of intermediate particle

- predominantly of one grain size, or having a range of sizes with some intermediate size missing.

Well Graded

Poorly graded

		<u> </u>			UNIFIED SOIL	CLASSIFICATION†
	MAJC	DR DIVI\$	IONS	GROUP SYMBOLS	TYPICAL NAMES	CLASSIFICATION CRITERIA
		f	CLEAN GRAVELS	GW	Well-graded gravels and gravel-sand mixtures, little or no fines	$\begin{array}{c c} & & & & & & & & & & & & & & & & & & &$
S,) sieve*	GRAVELS 50% or more of coarse fraction retained on No. 4 sieve	CLEAN	GP	Poorly-graded gravels and gravel-sand mixtures, little or no fines	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
1108 0	No. 200	GF 50% coar	GRAVELS WITH FINES	GM	Silty gravels, gravel-sand-silt mixtures	S N S S S S S S S S S S S S S S S S S S
AINE	ined on		GRAN WI	GC	Clayey gravels, gravel-sand clay mix- tures	Atterberg limits plot above 'A' line quiring use of dual symbols Atterberg limits plot above 'A' line quiring use of dual symbols
COARSE-GRAINED SOILS	More than 50% retained on No. 200 sieve ullet	coarse 4 sieve	CLEAN SANDS	sw	Well-graded sands and gravelly sands, little or no fines	and plasticity index greater than 7 Cu = D ₈₀ /D ₁₀ Greater than 6 Cu = D ₈₀ /D ₁₀ Greater than 6 Cc = D ₁₀ ×D ₆₀ Not meeting both criteria for SW Atterberg limits plot below 'A' line or plasticity index less than 4 Atterberg limits plot above 'A' line derline classifications requiring use of dual symptoms of the state
8	More th	SANDS More than 50% of coarse fraction passes No. 4 sieve	CLEAN	SP	Poorly - graded sands and gravelly sands, little or no fines	Not meeting both criteria for SW
		s ore than	DS FH ES	SM	Silty sands, sand-silt mixtures	Atterberg limits plot below 'A' line Atterberg limits plotting or plasticity index less than 4 in hatched area are bor.
		fra	SANDS WITH FINES	\$C	Clayey sands, sand-clay mixtures	Atterberg limits plot above 'A' line quiring use of dual symbols Atterberg limits plot above 'A' line quiring use of dual symbols
		AYS		ML	Inorganic silts, very fine sands, rock flour, silty or clayey fine sands	PLASTICITY CHART For classification of fine-grained soils and fine fraction of coarse
SOILS	50% or more passes No. 200 sieve"	SILTS AND CLAYS	Elquid ilmit 50% or less	CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays	soils and fine fraction of coarse- grained soils Atterberg limits plotting in hatched area are borderline classifications requiring use of dual symbols
FINE-GRAINED SOILS	passes No.	SILT		or.	Organic silts and organic silty clays of low plasticity	Equation of 'A' line: PI = 0.73(LL - 20) CL
FINE-GF	3% or more	CLAYS	50%	мн	Inorganic silts, micaceous or diato- maceous fine sands or silts, elastic silts	MH & OH
	25	SILTS AND CLAYS	greater than 50%	СН	Inorganic clay of high plasticity, fat clays	ML & OL
		SILT	gree	он	Organic clays of medium to high plasticity	0 10 20 30 40 50 80 70 80 90 100 LIQUID LIMIT
н	IGHLY	ORGANIC	SOILS	PT	Peat, muck and other highly organic soils	*Based on the material passing the 3 in. (75 mm) sieve †ASTM Designation D 2487, for identification procedure see D 2488

GROUND ICE DESCRIPTION

ICE NOT VISIBLE

GROUP SYMBOLS	SYMBOLS	SUBGROUP DESCRIPTION	
	Nf	Poorly-bonded or friable	
N	Nbn	No excess ice, well-bonded	
	Nbe	Excess ice, well - bonded	

NOTE:

- Duel symbols are used to indicate borderline or mixed fee classifications
 Visual estimates of ice contents indicated on borehole
- 3. This system of ground ice description has been modified from NRC Technical Memo 79, Guide to the Field Description of Permafrost for Engineering Purposes

LEGEND

Soil

fce T

VISIBLE ICE LESS THAN 50% BY VOLUME

GROUP SYMBOLS	8YMBOLS	SUBGROUP DESCRIPTION	
	Vx	Individual ice crystals or inclusions	1) ** 1 k
v	Vc	Ice coatings on particles	
	Vr	Random or irregularly oriented ice formations	
	Vs	Stratified or distinctly oriented ice formations	

VISIBLE ICE GREATER THAN 50% BY VOLUME

ICE	ICE + Soil Type	ice with soil inclusions	condition.
lue	ICE	ice without soil inclusions (greater than 25 mm (1 in.) thick)	Proposition of

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SNOWDRIFT G	RANULAR INVESTIGATI	ON	TESTPITS DUG WITI	1 D-6	BULLI	OOZE	R				PIT)609 -106(EBA	13
	EVELOPMENT CORPO	RATION	UTM ZONE: 12 NG	911300	1.00 F	5225	30.00						.00 (79	
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SAMPLE NO		SOIL DESCRIPT			<u> </u>					20	D-50 40 40 51LT 40 SANG 40 GRAVE	(%) 1 60 (%) 50	50 60 4)	SOIL SYMBOL	DEPTH (m)
0.0	PEAT (Pt),	and SILT light brown frozen								20	40	<u>60</u>	80		Pt ****	0.0
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SAMP IL TYPE SOIL DESCRIPTION SIT (ML) and SAND, fine light brown frazen GRAVEL (GW), some cobble some eard true sit median size 2 mm GRAVEL (GW), sity subtracted free water entering testpit End of tastpit 2.30 m.	GNWT TR	WN:	SPOF	TATS	ON	O HIIM OND BL							,		TPIT ject			0609 -106		BA	14
SOIL BESCRIPTION SOIL DESCRIPTION SOUND SOLUTION SOLUTION FORWEL (GW), some cobble some and trace sixt maximum size 40 mm subangular median size 2 mm SAND (SP), sitty solutroted free weter entering testpit End of testpit 2.30 m.																					
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5.0	-3.0				saturated free water entering test																-3.0
EDA Engineering Consultants Ltd. Completion Depth: 2.3 m Complete:	5.0		A 7		win a anim a Olas and Ila					in the second					1						-4.0
Yellowknife, N.W.T. LOGGED BY: TLJ FIG No: 10809-14 Page 1 of	Ł	ıΩ	A J	្សា		Lla.	\vdash					۷.১							I Po		

Date: 1992 04 08

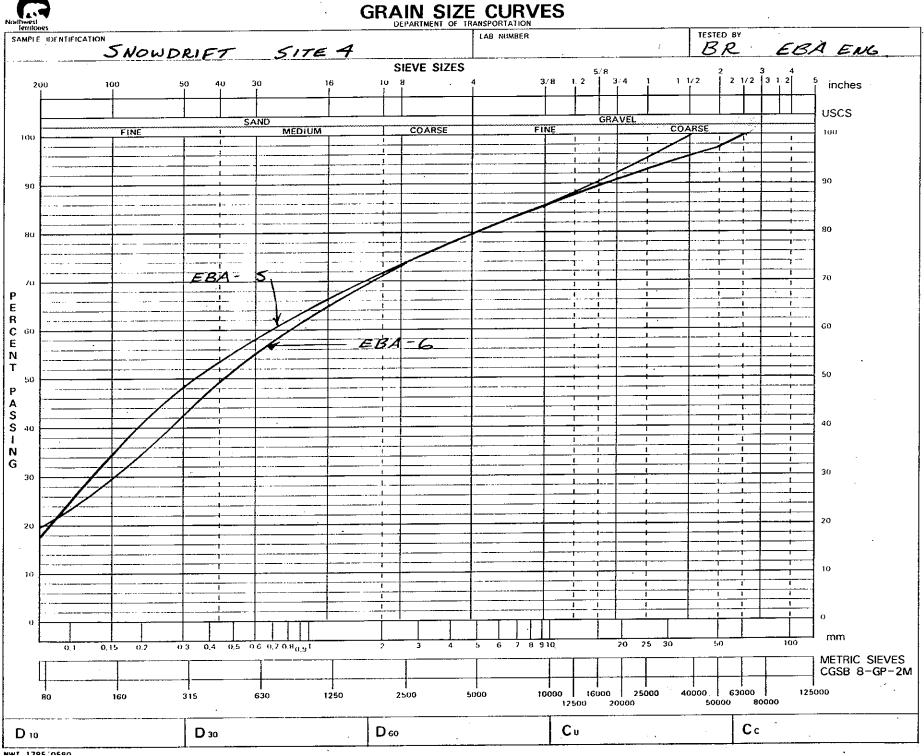
LABORATORY TEST RESULT SUMMARY

PROJECT TITLE: SNOWDRIFT GRANULAR INVESTIGATION

AREA	TESTPIT	GRAVEL (%)	SAND (%)	FINES (%)	CRUSH COUNT	L.A ABR. (%)	
			. 1.5			***************************************	
Site #7 Crush Sieve	14	67.0 64.0	33.0 35.0	0.4 1.1	68.0	16.0	
Site #7 Crush Sieve	13	74.0 66.0	25.0 32.0	1.4	80.0	17.0	
Site #7 Crush Sieve	12	69.0 65.0	30.0 33.0	1.3	80.0	16.0	
Site #6 Crush Sieve	11	28.0 31.0		0.3 0.5	20.0	***	
Site #6 Crush Sieve	10	47.0 54.0	52.0 45.0	0.8	58.0	17.0	
Site #6 Crush Sieve	9	65.0 56.0		1.0 1.5	67.0	17.0	
Site #4	6	21.0	59.0	20.0			
Site #4	5	21.0	61.0	18.0			

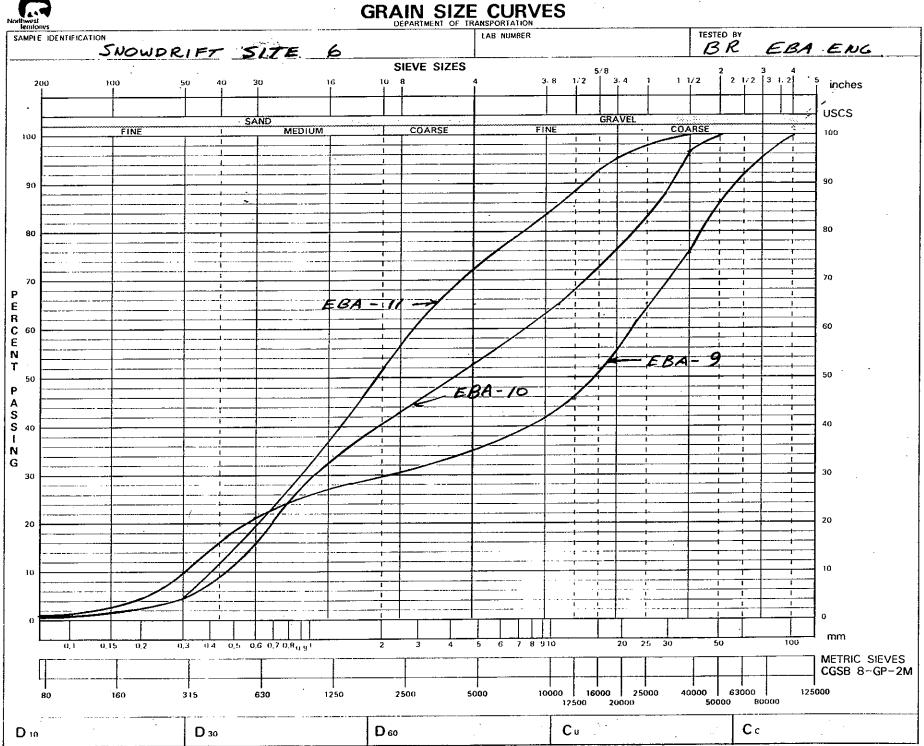
^{***} Insufficient sample size



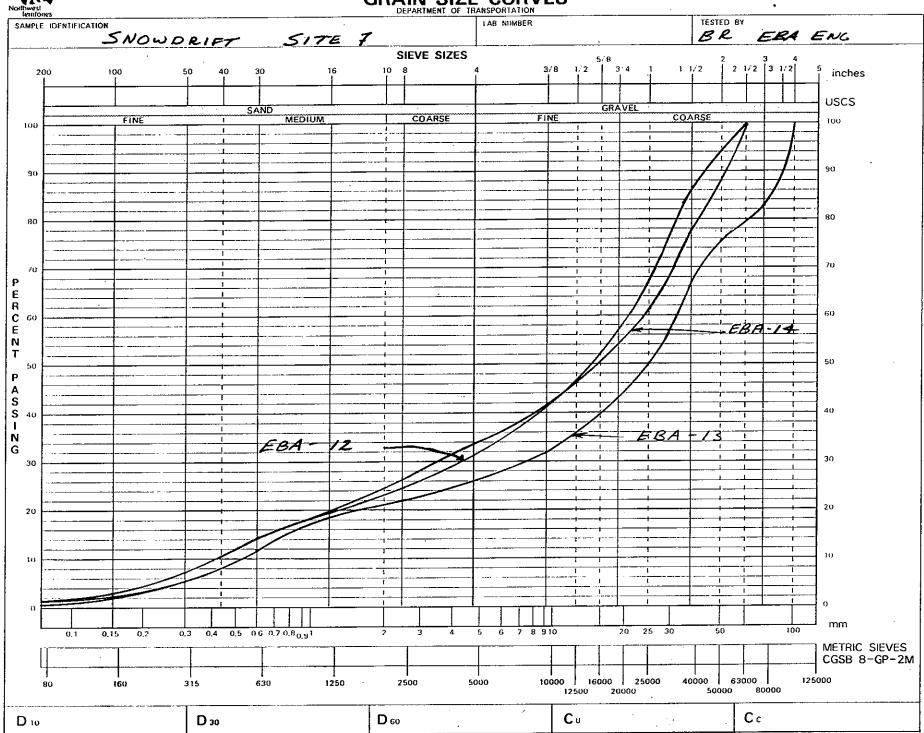




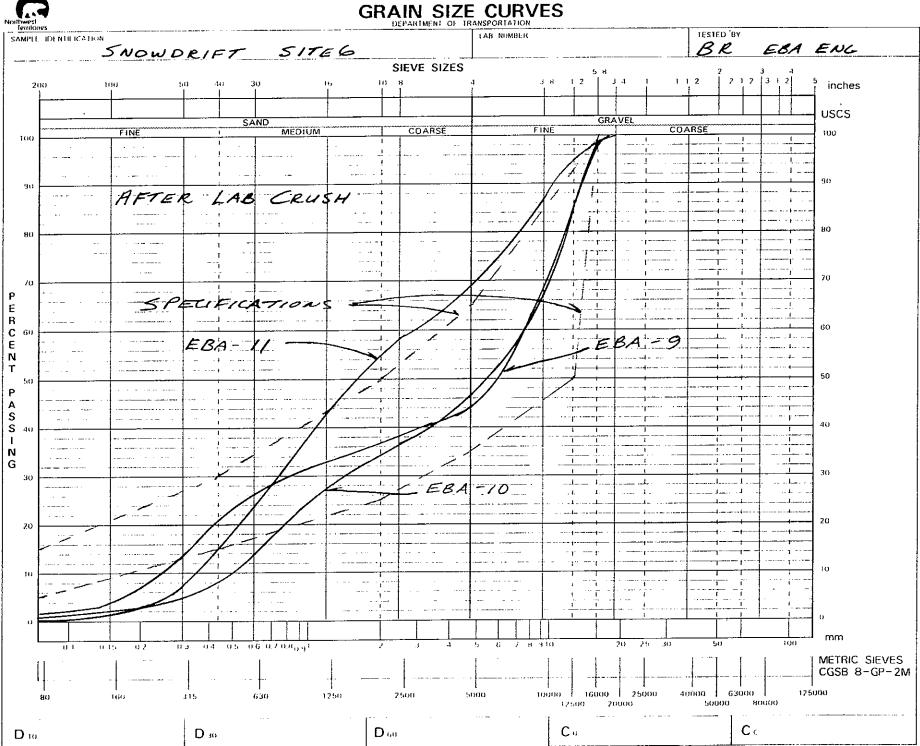
MWT 1795'0590



GRAIN SIZE CURVES
DEPARTMENT OF TRANSPORTATION

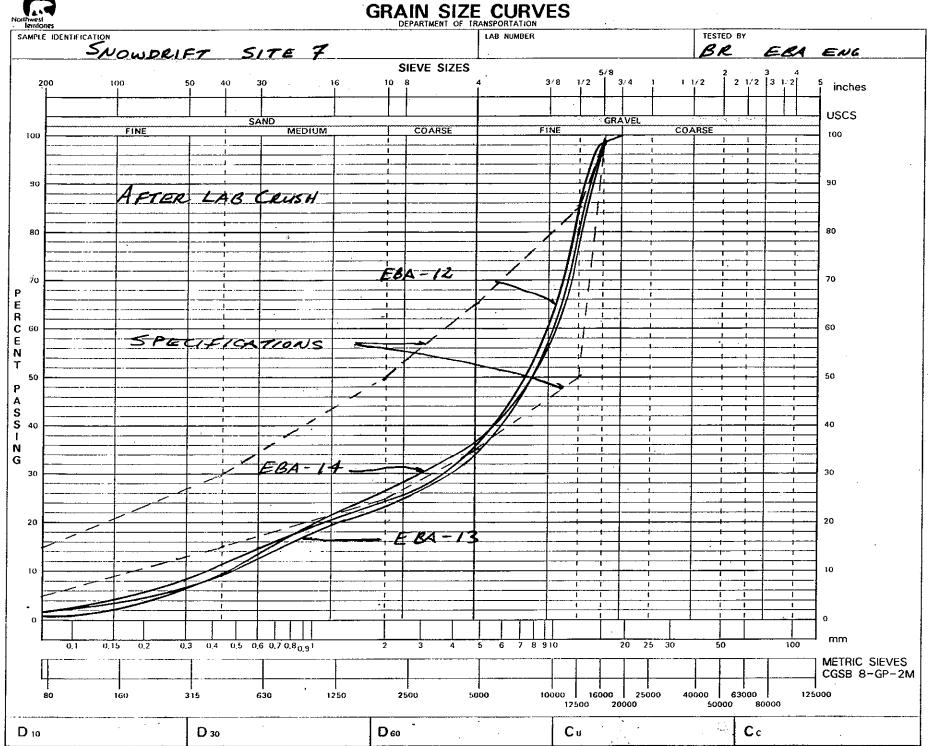


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APPENDIX C DESCRIPTION OF OVERLAND ROUTE



5+554

rough,

Snowdrift River Route (From T. Jordan's field notes March 18 - 19,1992)

LOCATION	ROUTE DESCRIPTION	VEGETATION & SOILS	РНОТО
Turn-off from air- port road 0+000	Route around lake is a flat lacustrine plain. After photo #1, rises gently about 2% to steep hill, about 8%. Then gradually rising at 1% for 200 m to steep hill (10% for about 30 m, then	Thick spruce to 10 m high.	# 1 , look- ing N # 2 look- ing N
	turning right about 4% grade for about 100m. 200 m long grade, average 4%,		look- ing S
	numerous local steep- enings. Then down hill about 3% for 100 m, then flat to left turn. Flat	Exposed shale on surface at summit.	
Traverse	for 400 m. Rising about 1% for 600 m. Sharp bend to right at	Trees a bit thinner, some tamarack. Black spruce to 4 m	
point #24 3+111	high point, then descend- ing at 3% for 100 m, then 8% for 100m.	high.	# 4 , look-
4+420	40 m high escarpment to right of road. Descending 1% for 600 m, then flat for 270 m.	Very thin peat cover over lt brown lacustrine silty clay.	ing S
	Descending 2% for 80 m to left turn, then descending 2% for 80 m to steep dip (about 1 m deep). Twisting for 50 m, while climbing to high point,		# 5 , look- ing N
	then descending at 2% for 80 m. Turn right. Undulating,	Beside lake on left.	



Traverse point #42 5+714

Turn right, then sharp left, cross creek, then climbing at 4% for 50 m, then 8% for 50 Curving to left, climbing 1% or less for 300m. looped right (off trail) to climb up onto shale ridge.

Lake on right

beside lake on right with

Turn right, flat occasional rising grade for 750 m, then turning right, flat with occasional descent for 600 m.

Sparse black spruce to 3 m.

Rising 5% for 100 m, then steeply over sandstone ridge about 1.5 m high, then descending at 2% for 100 m, then undulating for 500m.

Old burn area.

Stopped for tea

Nearly flat for 600 m. Left turn, due east, flat for 600 m. Turn right, climb 1% for 100 m, then 4% for 100 m onto sandstone ridge, then right, descending 4% for 50 m, then flat, turning right.

Thick spruce to 4 m, some birch to 5 m. Bay on left side.

Dead flat, due south for 300 m, then turn left, descending 4% for 100 m, then flat for 250 m.

Rising ground right of road.

Thick jackpine to 6 m high.

7, looking W

6 ,

looking N

Portage trail.

Drop through old river course, about 60 m wide. Banks about 6%. N bank 1.5 m high, S bank 2.0 m high.

Coarse angular and subangular sand in bank material. Dozer cleared trail to cabin on river for fuel pickup.



Approximately flat for 100 m, then climbing 6% for 70 m, then flat for 80 m. Looping right to climb up on dune ridge, then flat for 80 m, then dip and short steep climb Thick jackpine all (8%) out of dip. Then generally flat with some undulations for 200 m.

Steep short climb onto sandstone ridge, then jacent to road on flat with occasional climbing grade for 300 m. to road.

Turn right, then dead flat for 200 m.

Curve left for 300 m, dead flat, then descending 2 to 3% and slight meandering for 300 m, then dead flat, meandering for 600 m.

Short steep dip down at 8%, then undulating but generally descending, including one sharp dip through old water course about 1 m deep.

Still meandering about 600 m, then sharp rise onto dune edge, about 1.5 m high, then undulating and meandering 1 km through numerous apparent dune ripples.

the way from portage trail.

4 m high ridge adright, sloping down Thick black spruce to 8 m, some birch to 6 m. Almost no peat cover over lacustrine silty clay, some fine sand.

Thick jackpine to 6

Very thin peat cover over fine-medium silty sand.

Sand, fine-medium, angular, clean, may be glacial wash followed bу some dune action, but well-graded.



Turn off trail to gain acc- ess to area 6.

(About due east, dead flat about 500 m to river bank. Trail down to river is about 300 m at 8%, with some steeper descents, then about 300 m dead flat.)

8 , looki n g SW

Back on main trail at turn-off to area 6

Rough undulation, then climbing 8% for 20 m, then flat and curving to right, minor undulations in curve.

End of jackpine.
Fairly thick black
spruce to 6 m. Peat
cover thicker (about
100 mm).

Flat for 1 km, then curve left, then 700 m flat, to 5 m. curve left, 100 m, then right, slight rise onto jackpine ridge. Parallel river bank about 200 m, then swing left to descend at 5% for 200 m, then mainly flat parallelling river bank looking for way down.

Generally descending at 3% for 200 m to point where trail turns south-

Dense black spruce to 5 m.

3% for 200 m to point where trail turns southeast. We turn north and continue to descend at 8% for 300 m. Then turn left, climb over hump and continue down at 6% for 200 m to swamp, then flat, curving left around bedrock ridge, then downstream along shore-bound ice about 1 km to gravel bar (area 7)

Area 7



APPENDIX D

PHOTOGRAPHS





PHOTO #1 Looking north toward Snowdrift Airport, approx. sta. 1 +000. (Neg # 3A)



PHOTO #2 View looking north from approx. sta. 2+000. Snowdrift community is on extreme left, across lake. (Neg # 4A)

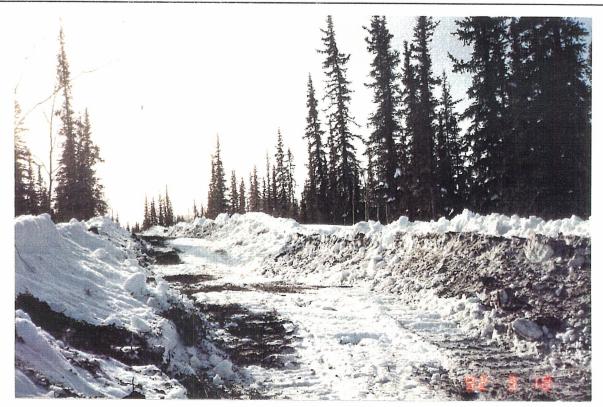


PHOTO # 3 View looking south from station: 2 + 400 (Neg # 5A)

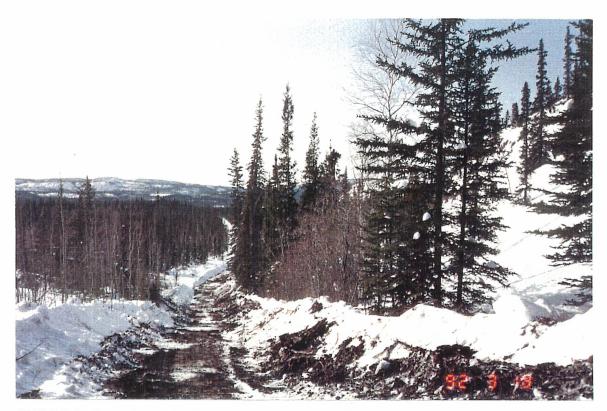


PHOTO # 4 View looking south from summit near station 4 + 100. (Neg # 6A)





PHOTO #5 View looking north from sta 5 + 300 (Neg # 7A)



PHOTO # 6 View looking north from approximately station 8 + 300 (Neg # 8A)



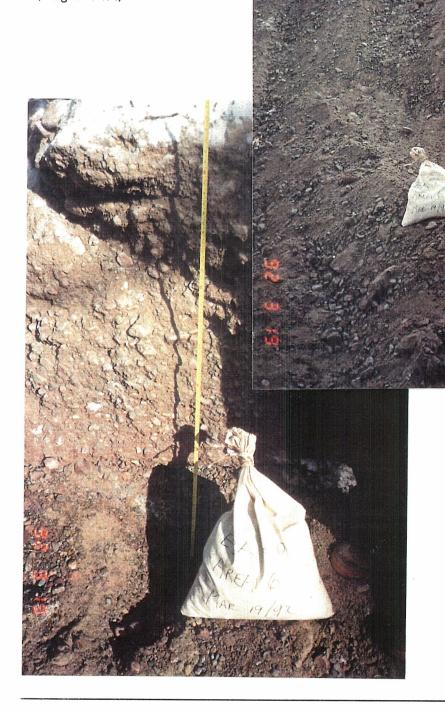


PHOTO #7 View looking west along E-W section north of Duhamel Lake. (Neg # 9A)



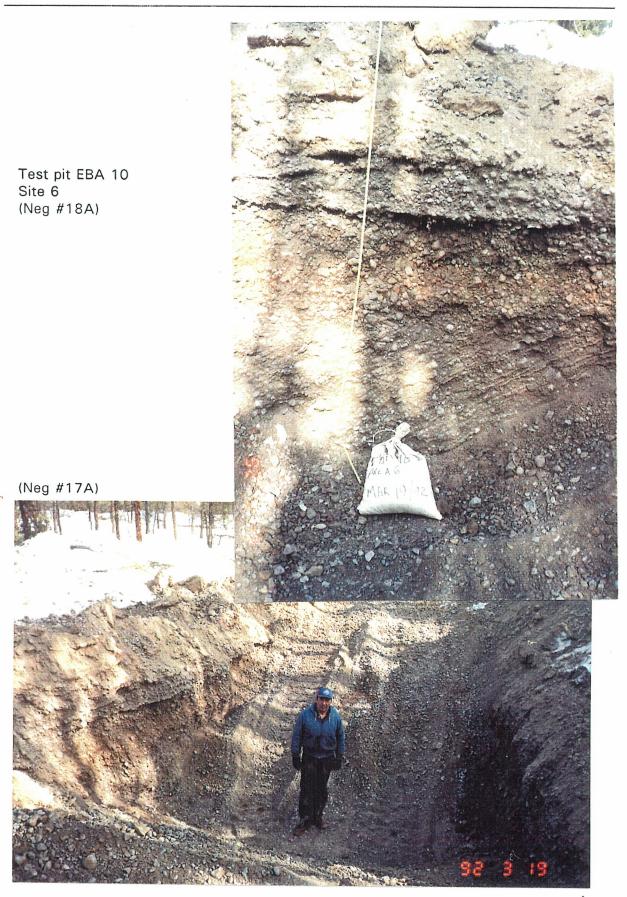
PHOTO #8 View looking South-east along sandy area between Duhamel Lake and Snowdrift River. (Neg # 10A)

Test pit EBA 9 Site 6 (Neg # 14A)

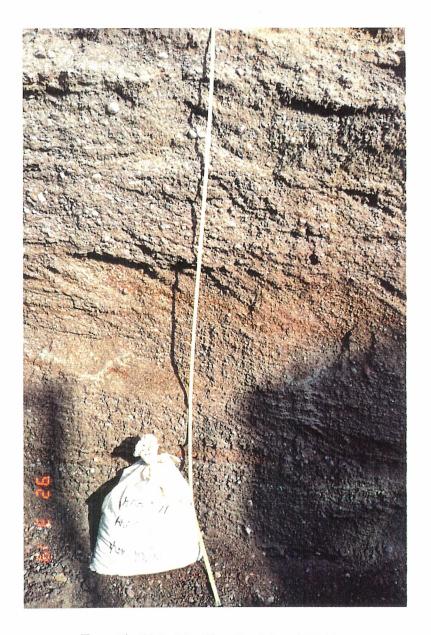


(Neg # 15A)









Test Pit EBA 11, Site 6. (Neg #20A)



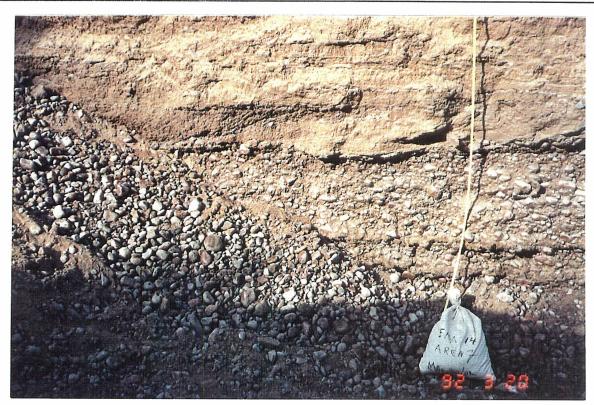


Test Pit EBA 13, Site 7. (Neg #00)

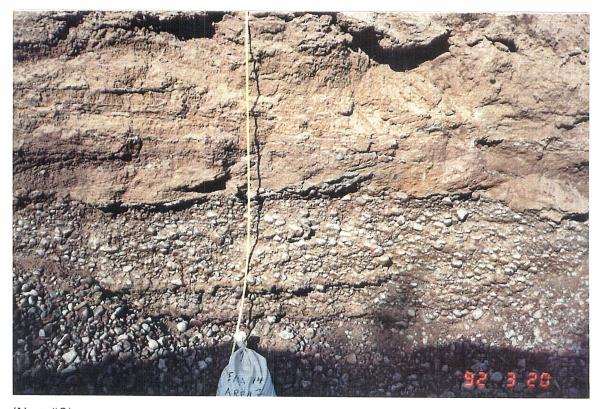


(Neg #24A)





Test Pit EBA 14, Site 7. (Neg #1)



(Neg #0)

