GEOTECHNICAL INVESTIGATION PROPOSED AIRSTRIP AND ACCESS ROAD FORT GOOD HOPE, NORTHWEST TERRITORIES

COPY



Keith Bornes

GEOTECHNICAL INVESTIGATION PROPOSED AIRSTRIP AND ACCESS ROAD FORT GOOD HOPE, NORTHWEST TERRITORIES

COPY

Prepared For

Government of the Northwest Territories

Department of Transportation

Yellowknife, Northwest Territories

Prepared By

HBT AGRA Limited

Yellowknife, Northwest Territories

YX00354 November, 1993



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1.0 <u>INTRODUCTION</u>

HBT AGRA Limited (HBT) was retained by the Government of the Northwest Territories, Department of Transportation (DOT), Airport Division Yellowknife, Northwest Territories to conduct a geotechnical investigation for a proposed airstrip and access road. A proposal to conduct this work dated July 23, 1993 was submitted to DOT. This work was authorized under Contract CT 933188.

1.1 SCOPE OF WORK

The scope of work for this project was as follows:

- determine the subsurface soils and the conditions at the proposed airstrip site and along the access road.
- identify and investigate potential granular sources for use in the construction of the airstrip and access road.
- conduct a laboratory testing program to determine the engineering properties of the soils.
- prepare a report outlining the results of the field work and laboratory testing, and present recommendations.

2.0 PROPOSED DEVELOPMENT

The proposed development includes the construction of an access road, approximately 3 km in length and the construction of an approximately 900 m long gravel surface airstrip with associated apron area. As part of the access road construction, a bridge is to be constructed at Jackfish Creek, and a culvert is to be installed on a stream at Curve 3 (Station 1+583 approximately).



3.0 FIELD AND LABORATORY INVESTIGATION

3.1 FIELD PROGRAM

The field work was conducted from September 29, to October 2, 1993. Twenty-nine boreholes were drilled along the access road alignment, airstrip alignment, and offset from the airstrip. The depth of boreholes ranged from 4.0 to 10.5 m below existing grade. The boreholes were drilled using a track-mounted B61 auger rig contracted from Mobile Augers and Research Ltd. The field work was supervised by Mr. J.L. Anklewich, P.Eng. of HBT. The subsurface conditions were logged on a continuous basis as drilling proceeded. Disturbed soil specimens were collected from most boreholes. In selected boreholes undisturbed samples of permafrost soils were recovered for inspection and photographing. Borehole locations are shown on the site plan, Figure 1. All boreholes were backfilled upon completion.

3.2 LABORATORY TESTING

Specimens retrieved during the drilling program were returned to HBT's laboratory for testing and classification to determine the engineering properties of the soils. The following tests were conducted on selected specimens:

- natural water content
- Atterberg Limits
- grain size analysis

The results of the laboratory tests are presented on, and following, the borehole logs in Appendix B.

4.0 SURFACE AND SUBSURFACE CONDITIONS

4.1 SITE DESCRIPTION

Access Road

The access road alignment had a cleared width of 20 to 30 m. The road begins on the east side of the community and traverses Jackfish Creek approximately 1500 m upstream of its confluence with the MacKenzie River. The north slope of the Jackfish valley has a maximum angle of about 16°, but along the alignment is about 12.5°. The south slope has angles up to 16.5° locally, but along the road alignment the overall angle is about 6°.

Vegetation consists of a mixture of coniferous and deciduous trees. Spruce and pine trees, up to about 10 m are the most common coniferous species and poplar/birch trees are the most common deciduous species, with heights up to about 7 m.

At Station 1+583 (approximately) the access road traverses a seasonal stream.

Selected site photographs of the access road are presented in Appendix A.

<u>Airstrip</u>

The airstrip had been grubbed approximately 120 m wide along its alignment.

The airstrip slopes slightly downward from the apron area to the west and east ends. The maximum grade change from the apron area to the west end is about 6.5 m.



At the west end of the airstrip the predominant tree cover was of coniferous species. Tree heights were up to about 8 m. Deciduous species became present and the total tree density increased to the east from a point about 200 m from the west end of the airstrip. Figures 4 and 5 present photographs of the airstrip at the time of the field investigation. There was a light snow cover. Access for the drill rig was reasonably good.

4.2 RELATED STUDIES

In addition to the geotechnical drilling conducted at this site, HBT has undertaken drilling for several other projects in the community.

- Geotechnical Investigation Health Centre, Fort Good Hope, NWT.
 Prepared for the Department of Government Services and Public Works, Inuvik, Northwest Territories.
- Geotechnical Investigation Subdivision Roadway, Fort Good Hope, NWT. Prepared for the Department Government Services and Public Works, Inuvik, NWT.
- Geotechnical Investigation School Addition, Fort Good Hope, NWT.
 Prepared for the Department of Government Services and Public Work, Inuvik, NWT.

Other reports also reviewed to obtain relevant information were:

 Granular Resource Inventory - Proposed Mackenzie Highway. Prepared for the Government of Canada by Pencam Services.



4.3 PERMAFROST CONDITIONS

The community of Fort Good Hope lies on the Arctic Circle, and on the physiographic boundary between continuous and discontinuous permafrost. Permafrost is likely to occur on highlands (away from streams and rivers) where the natural vegetative cover is intact. Normal seasonal thaw is likely limited to 0.5 m to 1.5 m below grade. In areas where the vegetation cover has been disturbed or removed, the seasonal thaw may be much greater and permafrost degradation can occur. In well drained granular soils, the seasonal that may be greater than 1.5 m.

The mean annual air temperature in Fort Good Hope is -7.8°C. Ground temperature information is scarce in the Fort Good Hope Area. A thermistor string installed at the location of the proposed health centre measured the following temperatures with depth in early October, 1993.

Depth Below Ground Surface (m)	Temperature (°C)
0	+0.7
1	+0.5
2	-0.1
3	-0.5
4	-0.8
5	-0.9
6	-0.8
7	-1.0
8	-1.1
9	-1.2

This data, measured in fine grained soils indicate that the permafrost is relatively warm, and potentially thermally unstable.

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4.4 SUBSURFACE CONDITIONS

Boreholes logs are presented in Appendix B. An explanation of the terms and symbols used in the logs is provided in Appendix C.

North Slope of Jackfish Creek - Station 2+860 to Station 3+120

Three boreholes (1,2, and 3) were advanced on the north slope of Jackfish Creek. The subsurface conditions encountered in Borehole 1 consisted of sand and/or gravels throughout its entire depth of 8.5 m. The soil was thawed to a depth of 2.1 m. The sand was generally coarse grained gravel, with sizes up to about 40 mm was present from about 2.4 m to 7.5 m depth. Ice crystals were encountered at a depth of about 5 m.

In Boreholes 2 and 3, sand, overlying clay till was encountered. The sand was silty, fine grained with some gravel sizes. At 0.9 m and 1.5 m in Boreholes 2 and 3 respectively clay till was encountered. The soil was unfrozen to a depth of 2.1 m. The clay was firm when thawed, medium plastic, containing some silt and coarser grained materials. Ice crystals were common throughout the clay till.

Jackfish Creek

Boreholes 4 and 5 were drilled on the north and south sides of the creek, respectively. Cohesionless soils were encountered to a depth of 2.7 m to 3.3 m below ground surface. The soils were thawed throughout this strata. The soils consisted predominantly of sands and gravels with sizes up to 75 mm. Groundwater seepage was experienced below about 2 m.

Underlying the cohesionless soils, frozen clay till was noted. The strata was firm when thawed, medium plastic, with frequent sand and gravel particles. Drilling below about 4 m was difficult due to sloughing within the granular layer near surface.

Access Road - Jackfish Creek to Airstrip

Boreholes 6 to 17 were drilled along the alignment of the access road. The depth of drilling ranged from 4 to 5.5 m. The subsurface conditions generally consisted of sand, or sand and gravel for the entire depth, or underlain by clay till. The depth of thaw was variable, ranging in depth from about 2 m to 4 m. From Station 0+880 (Borehole 14) to the airstrip no fine grained soils were encountered in the boreholes.

<u>Airstrip</u>

Twelve boreholes (Boreholes 18 to 29) were drilled on the airstrip alignment and right-of-way. Boreholes 18 to 22, drilled on the airstrip centerline encountered predominantly cohesionless soils throughout their depth. In Boreholes 18, strata of sand, gravel and sand and silt was observed with descending depth. The silt strata appeared to be limited to the west end of the airstrip.

In Boreholes 19 to 22 the subsurface soils generally consisted of sand and gravel to a depth of 4.3 to 5.2 m. The sand was fine to coarse grained and the gravel had gravel sizes to 50 mm. Underlying the sand and gravel was a strata of fine to coarse grained sand. The thawed depth was variable, ranging from 0.6 to 6.0 m below ground.



5.0 GRANULAR RESOURCES

5.1 GENERAL

The proposed airstrip and apron are located on a significant volume of granular material. This source was identified during the 1970's as part of work associated with the proposed Mackenzie Highway and oil and gas pipeline routes. It represents the largest granular source in the Fort Good Hope area. Construction of the airstrip over the deposit may seriously inhibit its full potential development. Consideration should be given to the impact of the construction of the airstrip on other project requirements for granular materials.

It is estimated that the total volume of granular soils in this deposit exceeds 1,000,000 m³. Within the upper 3 m, where the most suitable sand and gravel materials may be present, the estimated volume is 300 000 m³ to 500 000 m³.

Grain size curves for samples collected during the field program are presented in Appendix B. The natural material surrounding the airstrip is very good quality for embankment fill. However, the material encountered is unsuitable for crushing to produce the required volume of surfacing aggregate.

Inquiries were made of the availability of suitable crush source material near the community. Apparently there is no supply of oversize material (cobbles, boulders) that have been stockpiled at existing borrow pits. It may be necessary to consider using limestone from the nearby Ramparts.

5.2 BORROW SOURCE DEVELOPMENT

It is considered that the south side of the airstrip could be developed as a borrow source for granular soils. The eastern half of the site may require less overburden removal than the west end. As shown on the summary gradation curves in Appendix B the deposit is generally uniform with depth, but the upper 3 m of material may be somewhat more suitable than those at greater depths.

The thickness of overburden that will have to be removed to access the underlying granular soils may be estimated from the borehole logs. In most cases the organic layer is very thin, likely less than 200 mm. In a number of boreholes, such as Borehole 18, 22 and 28 a surficial layer of silt, up to 1.2 m thick, was encountered. Surficial silt should be expected to occur anywhere within the borrow source area.

The observed depth of thaw is variable, but deeper than normal. The depth to the permafrost was generally 3 to 4 m, or more. The natural water content was found to be less than 5% in most cases. This suggests that excavation of the borrow material should not be unduly hampered by ice rich (bonded) frozen granular soils.

6.0 <u>SLOPE STABILITY - JACKFISH CREEK</u>

One issue requiring serious consideration is the stability of the proposed alignment on the slopes facing Jackfish Creek. Degradation of permafrost will occur as a result of the clearing of the right-of-way and roadway construction. As this permafrost thaws the stability of the slope could decrease, if the soils are ice-rich and an increase in porewater pressures occur during the thawing process.



The north slope is likely to experience greater, and more rapid thaw than the south slope due to the greater exposure to the sun.

The soils on the north slope, and in particular in Boreholes 2 and 3 are predominantly clay till. The water content ranges from about 15% to 20%, with one single water content over 30%. These values indicate that the soil is not ice rich, and that thawing should not result in high porewater pressures. This is consistent with much of the clay till encountered in the Mackenzie Valley.

The north slope along the road alignment is about 12.5°, although the steepest grade is about 16°. These slope angles are considered to be thaw stable for ice poor clay tills, and consequently significant slope instability is not expected, based on the existing topography. The road alignment, however, crosses the contours at an angle and side slopes may pose a problem. Where significant earth works are envisioned (i.e. cuts and fills) as part of the road construction, the proposed section should be specifically evaluated from a slope stability perspective.

The south slope is in the order of 6° overall, along the road alignment. There is however a locally steep 15° section. As with the north side, stability of the slope during thawing is likely, providing grades are not steepened during construction.

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7.0 BRIDGE FOUNDATIONS

Two methods of bridge foundations are presented here. They are driven pile foundation and a crib foundation. Both are considered feasible.

It is understood that ice jamming may be a factor in establishing the elevation of the bridge deck, and in the design of the abutments. Although a study of the hydrology of Jackfish Creek is beyond the scope of this report, such a study could be undertaken by HBT who have recently conducted hydrographic studies of tributaries to the Mackenzie River.

7.1 DRIVEN STEEL PILES

Driven steel piles may be considered for use on this project. The allowable adhesion is derived from side bond between the steel and the frozen soil and thawing soil, and considers the potential for creep settlement. For this project, a ground temperature of -1°C has been used for design, and a creep settlement rate of 1.6 mm/year.

The capacity of driven steel piles in static compressive, or tensile loading, may be calculated on the basis of an allowable adhesion of 50 kPa, subject to the comments provided below.

The steel pile should consist of hollow structural steel sections, (not lacquered pipe steel), or H-sections. The section should be free of severe rust, scale, grease and lacquer. The upper sections may be painted with rust inhibiting paint.



Hollow section piles may be driven open-ended, with or without pre-drilling. Where pre-drilling is used to improve pile installation, the drill hole diameter should be at least 50 mm smaller than the pile diameter. H-pile sections should not be pre-drilled beyond a depth of 2 m below final grade.

The adhesion capacity of the pile within 2 m of the final ground surface and within any fill material should be ignored. The pile length should be embedded at least 7 m in natural soil. The top 2 m, in the seasonal active layer, should be covered with greased polythene sheets to reduce frost jacking forces.

The piles section should be selected with consideration of available pile driving equipment. Stresses during driving should not exceed approximately 40 percent of the yield strength of the pile section.

Where practical refusal is met during pile installation at a depth shallower than that specified by the design, a revised capacity may be established by review of the pile installation records.

For the purpose of lateral loading, the portion of the piles above a depth of 2 m below the final ground surface should be considered to be structurally unsupported.

7.2 CRIB FOUNDATION

A crib structure, placed in or on the banks of Jackfish Creek may be a suitable method of supporting the bridge deck. The design of the crib should consider issues such as bearing capacity, settlement, earth pressures and fill materials. A schematic of a crib is shown in Figure 6.



A crib constructed of timber or steel sections should be placed on a level surface with a bearing surface comprising of granular soils on clay tills. Silty soils, such as those encountered in Borehole 4 are not considered a suitable bearing soil. For a crib supported on compact to dense gravelly soil, or firm to stiff, ice-poor clay till, an allowable bearing capacity of 150 kPa may be used for design.

The lateral stability of the crib will depend on the earth pressures acting on it and the available frictional resistance along the base of the crib. The crib should be designed to resist an active force acting on the upslope side of the crib as follows:

$$P_{A} = 1/2\gamma' h^2 K_{A}$$

where P_A active force on the crib (kN/m width)

> effective unit weight of soil on upslope side of crib (assume 19 kN/m^3)

height of crib (m) h

active earth pressure (assume 0.33) K_A

The main resisting force will be derived from inter-particle friction along the base of the crib structure:

$$F_R = l\gamma' h \tan \phi$$

where F_R resisting force of crib (kN/m width)

length of the crib (m)

effective unit weight of crib fill (assume 20 kN/m3) γ

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height of crib (m) h



 ϕ = effective friction angle at crib - subgrade interface (assume 30°)

In calculating the stability of the structure, the ratio F_R/P_A should be at least 1.50.

The backfill used in the crib should consist of well compacted granular soils with a fines content of less than 10%. The fill should be compacted to at least 95 percent of Standard Proctor maximum dry density. The design of the crib should consider the earth pressures induced by the compacted fill and surcharge loadings due to construction, or traffic.

It is expected that the crib will experience some long term settlement due to permafrost degradation, fill settlement, and subgrade loading. Provisions should be included in the design to accommodate a settlement in the order of 50 mm. The approach to the crib may require periodic maintenance in light of vertical crib movement.

8.0 REVIEW AND INSPECTION

It is recommended that the geotechnical aspects of the final design be submitted to HBT for review and comments. This is to ensure that the design and proposed construction methods are consistent with the recommendations contained in this report.

It is also recommended that on-site monitoring and review be performed during construction to ensure that the subsurface conditions encountered are similar to those on which the design was based. This review is considered to be part of

the overall design process, and may be required by the National Building Code of Canada.

9.0 **LIMITATIONS**

Recommendations presented in this report are based on a geotechnical evaluation of the findings in 29 boreholes and a review of other available information. The subsurface conditions noted at the borehole locations have been assumed to be representative of the rest of the site. However, if conditions other than those reported herein are encountered during subsequent phases of this project, HBT should be notified and given the opportunity to review the recommendations in light of the new findings.

The subsurface investigation reported herein was performed for the purpose of providing foundation recommendations. An assessment of the subsurface conditions with respect to soil or groundwater contamination was beyond the scope of this report.

This report has been prepared for the exclusive use of the Government of the Northwest Territories for specific application to the development described in Section 2 of this report. It has been prepared in accordance with generally accepted soil and foundation engineering practices. No other warranty is made, either expressed, or implied.

10.0 CLOSURE

We trust this report meets your present requirements. We would be pleased to provide any further information that may be needed during the design and to

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advise on the geotechnical aspects for inclusion in contract documents. Should you require additional information or inspection services, please do not hesitate to contact our office.

Respectfully submitted

HBT AGRA Limited

James Anklewich, P.Eng. Manager, Yellowknife Office

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APPENDIX A

Figure 1
Access Road and Airstrip Alignments
Site and Borehole Location Plan

Selected Site Photographs

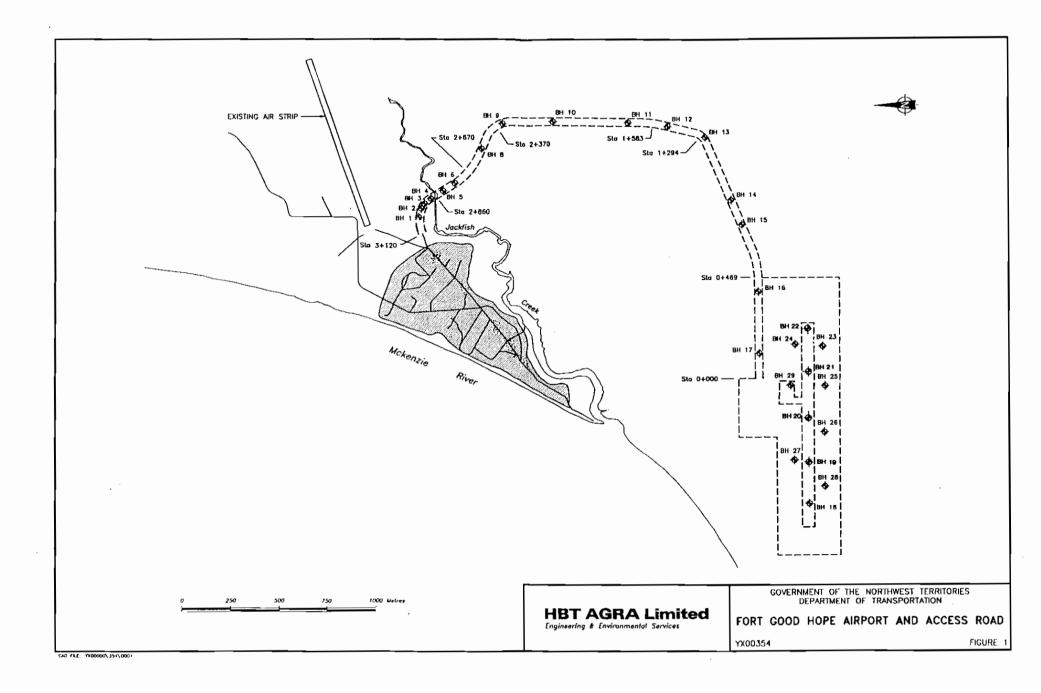




FIGURE 2: Fort Good Hope Access Road, Station 3+000 looking south towards Jackfish Creek



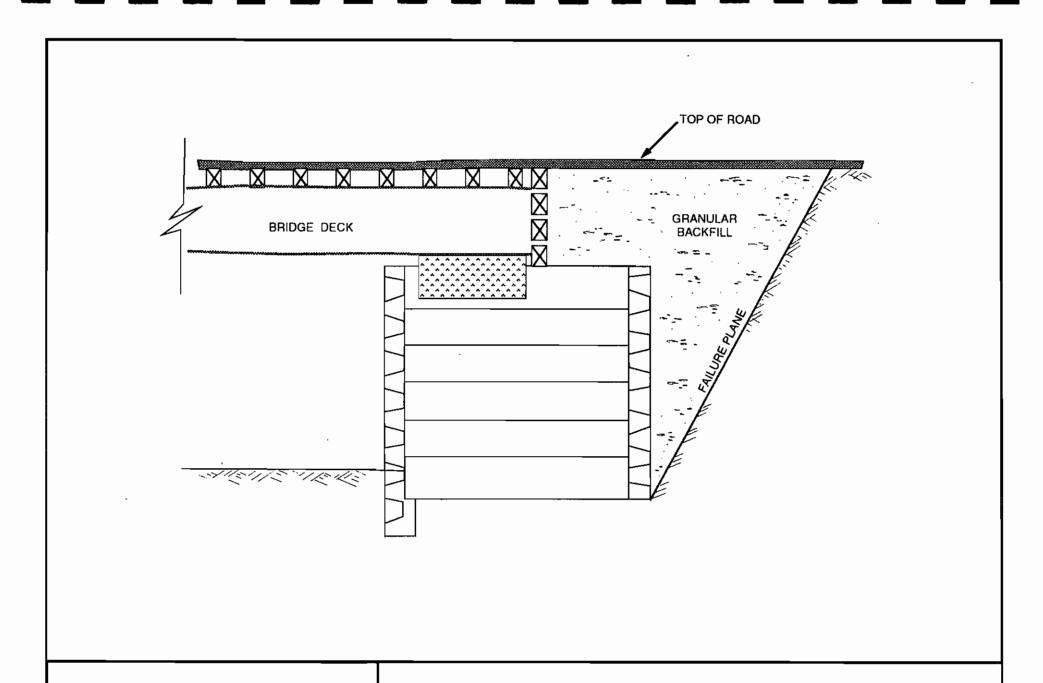
FIGURE 3: Fort Good Hope Airport Access Road looking north from Jackfish Creek. Note Borehole 4 on north side, and granular soils on north bank.



FIGURE 4: Fort Good Hope Airstrip at Station 3+220, looking west.



FIGURE 5: Fort Good Hope Airstrip, Station 3+880, looking east.



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Engineering & Environmental Services

E22AH01

SECTION THROUGH CRIB ABUTMENT **STRUCTURE**

FIGURE 6

APPENDIX B

Borehole Logs and Laboratory Test Results

PROJ	ECT: FOR	RT GOO	D HOP	PE AIRPO	RT	METHOD: TRACK MOUN	NTED AUGER		BOREHOLE No: 1	
				RANSPO				\rightarrow	Project No: YX00354	
				ERS AND) RESI			_	ELEVATION: 0.000 (m)	
SAMP	LE TYPE	_	DISTUR	RBED	<u> </u>	NO RECOVERY SPT	A-CASING [] 51	HELBY TUBE TORE	т —
DEPTH (m)	PLASTIC		I.C.		SOIL SYMBOL	SOIL DESCRIPTI	NOSTALIATION SAMPLE TYPE	SAMPLE NO		DEPTH (ft)
0.0	20	40	60	80	+-	SAND - loose, fine grained to r	medium	_		0.0
- 1.0	•					grained, trace of silt, dry yellowish brown — brown at 0.8m	y. 	D1		- - - - 5.0
- 2.0						- frozen at 2.1m - hard drilling at 2.3m SAND & GRAVEL		D2		[
- 3.0 -	•					 fine grained gravel, fine to coarse grained sand, silt, rounded, moist to w brown 	little	D3	Gravel = 39% Sand = 50%	- 10.0 -
- 4 .0 -						- coarse grained gravel (4	10mm)		Fines = 11%	- - 15.0
- 5 .0 -						ice crystals noticed in g5.0mwet	ravel at	D4		-
- 6.0 -										- 20.0 -
- 7.0 -	•							D5		- - - 25.0
- 8.0						SAND — medium to coarse grain little to some silt, trace gravel, moist, frozen, and	of 💮 💮	D6		-
- 9.0						End of Borehole at 8.5m				- - 30.0
- 10.0										-
- 11.0 -					,. 					- 35.0 - -
		<u> </u>	ЦD,	ጥ ለሮ	<u>Г</u>	Limited	LOGGED BY: JLA		COMPLETION DEPTH: 8.5 m	<u> </u>
						E, N.W.T.	REVIEWED BY: Fig. No:		COMPLETE: 93/09/29 Page 1	1 .4 4
				· · · · · ·	477	AND ATT IT A A .	IFRG. IND.		Page 1	i Of

PROJ	ECT: FORT	GOOD HOF	PE AIRPOR	et T	METHOD: TRACK MOUN	TED AUGER	8	OREHOLE No: 2	
	T: DEPARTI				N DATUM:		P	roject No: YX00354	
	RACTOR: M			_			_	LEVATION: 0.000 (m)	
SAMP	LE TYPE	DISTUR	BED	\perp	NO RECOVERY SPT	A-CASING [SH	ELBY TUBE TORE	1
ОЕРТН (m)	PLASTIC	M.C.	LIQUID	SOIL SYMBOL	SOIL DESCRIPTION	WELL NSTALLATION SAMPLE TYPE	SAMPLE NO	TEST COMMENTS	DEPTH (ft)
0.0	20	40 50	80		 SAND - top 200mm high in org	anics			0.0
-					loose, silty, fine grained medium grained sand, dry	to	D1		
- 1.0					CLAY (Till) — silty, sandy, gravelly, me plastic, damp to moist, g	dium reyish-	-		- 5.0
- 2.0					brown — frozen at 2.3m	·	D2		-
- 3.0									10.0
4.0					— ice crystals at 3.7m		D3		45.0
- 5.0					Permafrost core barrel at 4.9m - ice seam between 4.9-5	.5m	D4		- 15.0 -
6.0							C1		20.0
- 7.0	•				End of Borehole at 7.0m		D6		-
- 8.0									- 25.0 -
- 9.0									- - - 30.0
- 10.0									- -
- 11.0									35.0
_									-
		HB	ľ AGR	ZA		LOGGED BY: JLA		COMPLETION DEPTH: 7.0 m	<u> </u>
					E 37 177 E	REVIEWED BY: Fig. No:		COMPLETE: 93/09/29	1 of 1

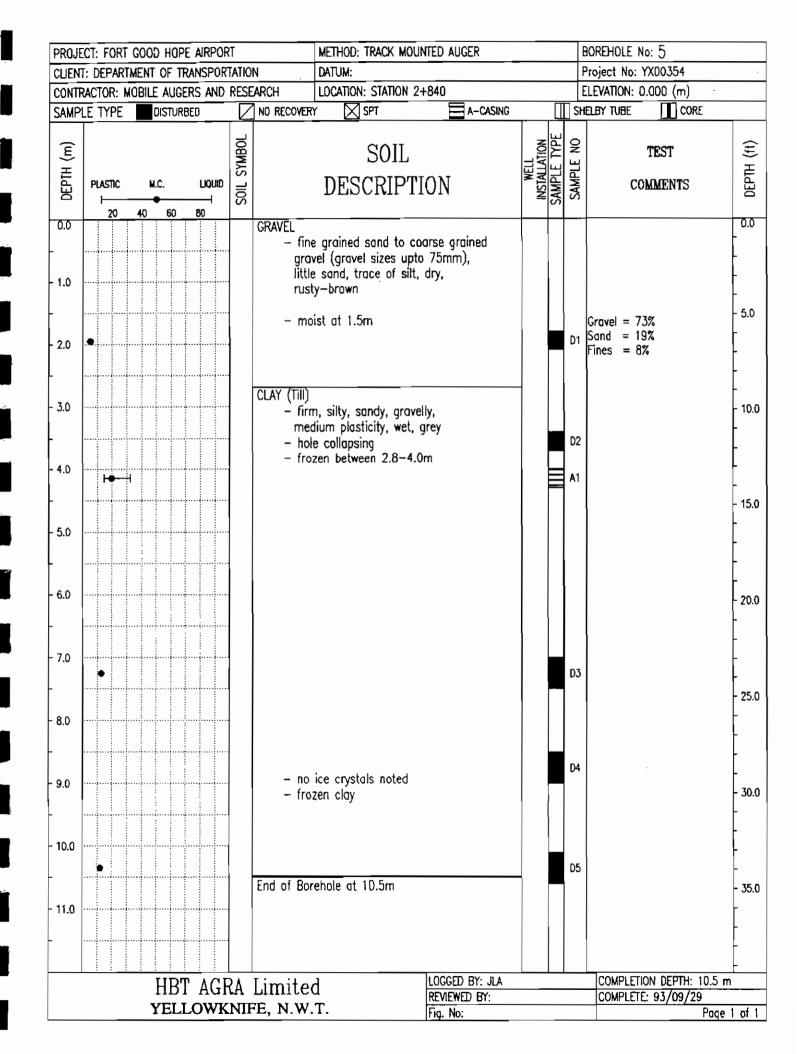
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PROJE	CT: FORT	GOOD HO	PE AIRPOR	 ₹T	METHOD: TRACK MOUNTED AUGER		E	OREHOLE No: 3	
	T: DEPARTA						_	Project No: YX00354	
	RACTOR: MO			RESE				LEVATION: 0.000 (m)	
SAMP	LE TYPE	DISTU	RBED	_ <u>L</u>	NO RECOVERY SPT BA-CASING	<u> </u>	II SH	ELBY TUBE CORE	
(m) +				SYMBOL	SOIL	WELL INSTALLATION AMPI F TYPF	Let	TEST	H (ft)
ОЕРТН (m)	PLASTIC	M.C.	FIĞNID	SOIL S	DESCRIPTION	WELI INSTALLA SAMPIF	SAMPLE	COMMENTS	DEPTH
0.0	20	40 60	80		SAND - top 200mm high in organics				0.0
_					 fine grained sand to coarse grained 				Ā
- <u>▼</u> - 1.0					gravel — little silt, wet, brown at 0.6m				
					- gravel seam between 0.9-1.5m				- 5.0
_					CLAY (Till) - silty, sandy, gravelly, medium				- 5.0
- 2.0				1	plastic, wet (water coming into		D1		-
					hole), greyish brown				-
					- collapsing hole	1			†
- 3.0				1					- 10.0
-				}					
- 4.0	•						D2		-
									-
-				1					- 15.0
- 5.0				ł			D3		
•]					-
- 6.0				1					- 20.0
-									+
						1	D4		†
- 7.0				1	- hole sloughing in		1		Į
-									- 25.0
- 8.0									-
2.0					- ice crystals between 7.9-8.5m		D5		-
-					End of Borehole at 8.5m	-			-
- 9.0									-
									30.0
-									_
- 10.0									-
_									-
									- 35.0
- 11.0									
-									
									-
		HB	T AGE	RA	Limited LOGGED BY: JLA		_	COMPLETION DEPTH: 8.5 m	
					FE, N.W.T. REVIEWED BY: Fig. No:			COMPLETE: 93/09/29	1 of 1

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PROJE	CT: FOR	COOD HO	PE AIRPO	रा		METHOD: TRACK MOUNTED AUGER BOREHOLE No: 4						
CLIENT: DEPARTMENT OF TRANSPORTATION						DATUM:	· \n	Project No: YX00354				
				RESE		LOCATION: STATION			_	_	LEVATION: 0.000 (m) LEY TUBE CORE	
SAMP	LE TYPE	DIST	HREED	<u> </u>	NO RECOVERY	∕ ⊠ SPT	A-CASING	т-	т <u>⊔</u>		IBI TOBECORE	1
(E)				SYMBOL		SOÌL	1	LL	TPE	E NO	TEST	(£)
ОЕРТН (m)	PLASTIC	M.C.	LIQUID	SOIL ST		DESCRIP	TION	WEL INSTALL	SAMPLE TYPE	SAMPLE	COMMENTS	DEPTH (ft)
- 00	20	40 60	<u> </u>	ļ °,	CUT AND C	AND -		_	J .			0.0
0.0					SILT-AND SA - 300	ANU mm of organics						-
<u> </u>						-				,,		
1.0					- 050	y at 1.1m				D1		
-					- soft	t, coarse grained						- 5.0
					mois GRAVEL	st, brown at 1.2-	1.8m	-}				-
- 2.0]	– san	dy, wet, brown						}
}				1	– hare	d drilling						
- 3.0												10.0
				.]	CLAY (Till)			_				-
						zen at 3.4m						
- 4.0				1		ehole at 4.0m du	e to sloughing					[
-					conditions.							- 15.0
- 5.0]								-
3.0					ļ							
t												
6.0												- 20.0
												}
								1				-
- 7.0				1				1	1	[
-	ļ			-				-	1			- 25.0
- 8.0										1		-
0.0									1			-
				1								
9.0												30.0
_												-
												-
- 10.0												
-	 			-								- 35.0
- 11.0												- 55.0
												-
												-
		TIT	ጋጥ ለር	D A	Limite		LOGGED BY: JLA		L		COMPLETION DEPTH: 4.0 m	ŀ
					Limited E, n.w.:		REVIEWED BY:				COMPLETE: 93/09/29	1 -4 1
1					-,		Fig. No:				rage	1 of 1

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PROJECT: FORT GOOD HOPE AIRPORT METHOD: TRACK MOUNTED AUGER BOREHOLE No: 6 DATUM: Project No: YX00354 CLIENT: DEPARTMENT OF TRANSPORTATION LOCATION: STATION 2+760 ELEVATION: 0.000 (m) CONTRACTOR: MOBILE AUGERS AND RESEARCH ∑ SPT A-CASING CORE NO RECOVERY SHELBY TUBE SAMPLE TYPE DISTURBED SAMPLE TYPE SYMBOL \equiv DEPTH (m) S0IL TEST DEPTH 0 **DESCRIPTION** PLASTIC LIQUID SOIL M.C. COMMENTS 60 80 20 0.0 0.0 ORGANICS - soft, silty sand, rootlets D1 SAND - soft, silty, damp, brownish grey D2 - frozen at 0.8m 1.0 CLAY (Till) 5.0 - silty, sandy, gravelly (40mm), medium plastic, dry 2.0 D3 - hard drilling between 2.7-3.0m 3.0 10.0 D4 4.0 End of Borehole at 4.0m 15.0 5.0 6.0 20.0 7.0 25.0 8.0 9.0 30.0 10.0 35.0 11.0 LOGGED BY: JLA COMPLETION DEPTH: 4.0 m HBT AGRA Limited REVIEWED BY: COMPLETE: 93/09/29 YELLOWKNIFE, N.W.T. Fig. No: Page 1 of 1

PROJE	CT: FORT	GOCD HO	PE AIRPOR	1	METHOD: TRACK MOUNTED AUGER			BC	OREHOLE No: 7	
			TRANSPOR					_	roject No: YX00354	
			GERS AND	RESE			ш		EVATION: 0.000 (m)	
SAMP	LE TYPE	DISTU	IRBED	\mathcal{L}	NO RECOVERY SPT A-CASING		ГЩ	SHE	LBY TUBE CORE	
DEРТН (m)	PLASTIC	M.C.	LIQUID	SOIL SYMBOL	SOIL DESCRIPTION	WELL	SAMPLE TYPE	SAMPLE NO	TEST COMMENTS	DEPTH (ft)
0.0	20	40 60	<u>80</u>	0,	CONTRACTOR OF THE PROPERTY OF		Ŭ,			0.0
0.0					SAND — loose, fine grained sand, trace of silt, dry, brownish grey			51		- 0.0
- 1.0					- medium grained sand, trace to little silt at 1.1m			D1		- - - 5.0
- 2.0					– frozen at 1.2m			D2		-
- 3.0										10.0
- 4.0					End of Borehole at 4.0m	-		D3		<u> </u>
- 5.0										- 15.0 -
- 6.0									·	- 20.0
- 7.0										-
- 8.0										- 25.0 -
9.0										- 30.0
- 10.0										-
- 11.0										- 35.0 L
		<u>וון</u>	<u>፡፡፡፡</u>) A	Limited LOGGED BY: JLA		LL		COMPLETION DEPTH: 4.0 m	<u> </u>
					REVIEWED BY:				COMPLETE: 93/09/29	
		I EL	FOWK	MIP.	E, N.W.T.				Page	1 of 1

PROJ	ECT: FORT	GOOD HOP	PE AIRPOR	T	METHOD: TRACK MOUNTED ALIGER			В	OREHOLE No: 8	
		MENT OF T						_	roject No: YX00354	
		OBILE AUG		RESE			_		LEVATION: 0.000 (m)	
SAMP	LE TYPE	DISTUR	RBED		NO RECOVERY SPT A-CASING		Щ	SHE	LBY TUBE TORE	
DEРТН (m)	PLASTIC	M.C.	LIQUID	SOIL SYMBOL	SOIL DESCRIPTION	WELL	SAMPLE TYPE	SAMPLE NO	TEST COMMENTS	DEPTH (#)
0.0	20	40 60	80		SAND — loose, fine grained sand, trace of	+	1		<u> </u>	0.0
1.0					silt, dry, yellowish brown — little silt and moist at 0.9m			D1		-
2.0					 frozen at 1.5m medium grained sand, little silt 			D2		- 5.0 -
3.0										- 10.0
4.0					End of Borehole at 4.0m	1				15.0
5.0										-
6.0										20.0
7.0										- - - 25.0
8.0										- - -
9.0										- 30.0
10.0										-
11.0										- 35.0 - -
		LID	<u>፡ ፡ ፡</u> ጥ ለሶር) A	Limited LOGGED BY: JLA		_		COMPLETION DEPTH: 4.0 m	
					REVIEWED BY:				COMPLETE: 93/09/29	
		Y EL.	LOWK	III	FE, N.W.T.				Page	1 of 1

CLEWIT DEPARTMENT OF TRANSPORTATION DATUM: CONTRACTOR MOBILE AUGIERS AND RESEARCH LOCATION STATION 2+350 ENAMPLE TYPE SIGNERGED SOUL SOURCE MAY BE SOUND TO SO THE STATION 2+350 SOUL DESCRIPTION SOUL DESCRIPTION SOUL DESCRIPTION SOUND SOUND THE STATION STATION 2+350 DESCRIPTION SOUL DESCRIPTION SOUND	PRO.II	ECT: FORT	GOOD HO	OPE AIRPOF	श	METHOD: TRACK MC	DUNTED AUGER			BC	DREHOLE No: 9	
CONTRACTOR: WIGNER AUGERS AND RESEARCH SAMPLE TYPE SOURCE PAUSIC: No. C. UGUID TO 40 90 80 SAND - fine grained sand, trace of sit dry, brown - sity, low plasticity, and moist at 1.7m - medium grained sand, little to some sit dry, brown - frozen at 1.7m - medium grained sand, little to some Sittle to some End of Borehole at 4.0m - 10.0 - 3.0 HBT AGRA Limited LOGGED BF: JUA COMPLETION DEPTH: 4.0 m HBT AGRA Limited LOGGED BF: JUA COMPLETION DEPTH: 4.0 m COMPLETION DEPTH: 4.0 m REMEMBE BF: COMPLETION DEPTH: 4.0 m COMPLETION DEPTH: 4.0 m COMPLETION DEPTH: 4.0 m REMEMBE BF: COMPLETION DEPTH: 4.0 m COMPLETION DEPTH: 4.0 m COMPLETION DEPTH: 4.0 m REMEMBE BF: COMPLETION DEPTH: 4.0 m COMPLETION DEPTH: 4.0 m COMPLETION DEPTH: 4.0 m COMPLETION DEPTH: 4.0 m REMEMBE BF: COMPLETION DEPTH: 4.0 m										_		
SOIL DESCRIPTION Solution Description Descriptio	CONT	RACTOR: MO	BILE AU	IGERS AND	RESE	ARCH LOCATION: STATION	2+360			_		
DESCRIPTION	SAMP	LE TYPE	DIST	URBED	Z	NO RECOVERY SPT	A-CASING			SHE	LBY TUBE CORE	
SAND - fine grained sand, trace of silt dry, brown D1	ОЕРТН (т)	⊢	—•	——	SOIL SYMBOL		TION .	WELL INSTALLATION	SAMPLE LIPE	SAMPLE NO		DEPTH (ft)
100	0.0	20	40 60	3 80	┼	SAND - fine grained sand, tr	race of silt	-	t	_		0.0
- medium groined sond, little to some	- 1.0					- silty, low plasticity, ar at 1.2m	nd moist			D1		- 5.0
End of Borehole at 4.0m -15.0 -6.0 -7.0 -8.0 -10.0 -	- 2.0 -					- medium grained sand	l, little to some .			D2		- - -
-5.0 -5.0 -5.0 -5.0 -5.0 -5.0 -5.0 -5.0	-									D3		- 10.0 -
-6.0 -7.0 -8.0 -9.0 -10.0 -10.0 -30.0 -30.0 -35.0 -35.0 -35.0 -35.0 -35.0 -35.0 -35.0 -35.0 -35.0 -35.0	-					End of Borehole at 4.0m	-					- - 15.0
-7.0 -8.0 -9.0 -10.0 -11.0 -30.0 -11.0 -35.0 -35.0 -36.0 -36.0 -36.0 -37.0 -38	-											- - -
- 25.0 - 25.0 - 30.0 - 10.0 - 11.0 - 11.0 - 35.0 - 35.0 - 11.0	-											- 20.0 -
HBT AGRA Limited LOGGED BY: JLA REVIEWED BY: COMPLETION DEPTH: 4.0 m REVIEWED BY: COMPLETE: 93/09/29	-											- 25.0
HBT AGRA Limited LOGGED BY: JLA REVIEWED BY: COMPLETION DEPTH: 4.0 m REVIEWED BY: COMPLETE: 93/09/29												} - -
HBT AGRA Limited LOGGED BY: JLA REVIEWED BY: COMPLETION DEPTH: 4.0 m COMPLETE: 93/09/29	-											- 30.0 -
HBT AGRA Limited LOGGED BY: JLA COMPLETION DEPTH: 4.0 m REVIEWED BY: COMPLETE: 93/09/29	10.0											- - 35.0
REVIEWED BY: COMPLETE: 93/09/29	- 11.0 -											- - -
REVIEWED BY THE REVIEWED BY THE REVIEWED BY THE PROPERTY OF TH			HF	BT AGE	RA	Limited						
												1 of 1

PROJE	CT: FORT	GOOD HO	PE AIRPOR	ī	METHOD: TRACK MOUNTED AUG	ER	BORE	HOLE No: 10	
	T: DEPARTM			_			_	ect No: YX00354	
	RACTOR: MI			_				ATION: 0.000 (m)	
SAMP	LE TYPE	DISTU	RBED	\angle	NO RECOVERY SPT	A-CASING []]	SHELBY	TUBE CORE	
ОЕРТН (m)	PLASTIC	M.C.	LKQUID	SOIL SYMBOL	SOIL DESCRIPTION	WELL INSTALLATION SAMPLE TYPE	SAMPLE NO	TEST COMMENTS	DEPTH (ft)
0.0	20	40 60	80		SAND - 250mm of organics		-		0.0
- 1.0					trace of silt, dry, light brownblack grey silt seams		D1	•	-
-	•				 fine to medium grained sand, do to moist frozen at 1.5m 	· 1 🗀	02		- 5.0
- 2.0	•				CLAY — silty, medium plastic, frozen, greyish brown		03		-
- 3.0									- 10.0
- 4.0	•				- ice crystal noted between 3.4-4 End of Borehole at 4.0m	.0m	D4		-
- 5.0					·				- 15.0 -
- 6.0									20.0
- 7.0									-
- 8.0								·	- 25.0 -
- 9.0					•				- 30.0
- 10.0									
- 11.0									- 35.0
-									-
		HB	T AGF	RS	Limited LOGGED REVIEWEL			OMPLETION DEPTH: 4.0 m OMPLETE: 93/09/29	
		YEL	LOWK	NII	FE, N.W.T. Fig. No:	, <u>, , , , , , , , , , , , , , , , , , </u>			1 of 1

PROJE	CT: FORT	GOOD HO	PE AIRPOR	T	METHOD: TRACK MOUNTE	ED AUGER	BOREHOLE No: 11
			TRANSPOR				Project No: YX00354
			gers and	RESE			ELEVATION: 0.000 (m)
SAMP	LE TYPE	DISTU	IRBED	\mathcal{L}	NO RECOVERY SPT	A-CASING [SHELBY TUBE CORE
ОЕРТН (m)				SYMBOL	SOIL	ELL LATION E TYPE	ON TEST (1) HLDGO
DEPT	PLASTIC	M.C.		SOIL S	DESCRIPTIO	N WEL	COMMENTS COMMENTS
0.0	20	40 60	80		SILT — soft, sandy, low plastic, m	noist,	0.0
-					and beige		D1 [
- 1.0							-
['.0					– wet at 1.2m		
<u> </u>							- 5.0
- 2.0							02
				Į			-
							\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
- 3.0			- i - i - i - i - i - i - i - i - i - i				- 10.0
}						1 [}
- 4.0						1 1	-
					- frozen at 4.10m		
							- 15.0
5.0					SAND - silty, medium grained sa	nd, frozen,	
					brown, low plasticity		_
					End of Borehole at 5.5m		} <u> </u>
6.0							- 20.0
-							
7.0]
							-
							- 25.0
- 8.0							
-							
							-
- 9.0							- 30.0
-							
10.0							
							-
							- 35.0
- 11.0							
-							
						·	
					rimired E	OGGED BY: JLA REVIEWED BY:	COMPLETION DEPTH: 5.5 m COMPLETE: 93/09/29
		YEL	LOWK	NIF		ig. No:	Page 1 of 1

INOUL	.C1: FORI	GOCD HO	DPE AIRPO	रा <u> </u>			MOUNTED AUG	ER			_	IOREHOLE No: 12	
			TRANSPOR			ATUM:			_		_	roject No: YX00354	
			GERS AND	RESI		DCATION: STATIC				ГП	_	LEVATION: 0.000 (m)	
SAMPL	LE TYPE	DIST	JRBED	<u> </u>	NO RECOVERY	SPT	=	A-CASING	1	<u>Щ</u>] Shi	ELBY TUBE CORE	T
DEPTH (m)	PLASTIC	M.C.	LIQUID	SOIL SYMBOL]	SOI DESCRII			WELL	SAMPLE TYPE	SAMPLE NO	TEST COMMENTS	ОЕРТН (ft)
0.0	20	40 60	80	-	SAND - 250r	nm of organic	cover			\vdash			0.0
- 1.0				 -	- silty, SILT - soft, brown	<u>black</u> sandy, low pla					D1 D2		- - -
- 2.0					- frozer GRAVEL - sa brown CLAY - silty,			7en			52		- 5.0 -
- 3.0				 	OLNI SIKY,	medidir riigi	plastic, not						- 10.0
- 4 .0 -					End of Boreh	ole at 4.0m							- - - 15.0
- 5.0 -													-
- 6.0													- 20.0 -
- 7.0													- 25.0
- 8.0													-
- 9.0													- 30 .0
- 10.0													- - - 35.0
- 11.0						_							- - -
		HE	BT AGI	RA	Limited		LOGGED REVIEWE		_			COMPLETION DEPTH: 4.0 m COMPLETE: 93/09/29	
					E, N.W.T.		Fig. No:						1 of 1
				_								<u> </u>	

PROJE	CT: FOR	T GOCD H	OPE A	IRPOR	T T	METHOD: TRACK MOU	INTED AUGER			E	OREHOLE No: 13	
		RTMENT OF			_					_	roject No: YX00354	
		MOBILE AL			RESE				Ш		LEVATION: 0.000 (m) ELBY TUBE TO CORE	
SAMPL	LE TYPE	UIST	URBED			NO RECOVERY SPT	A-CASING	ГТ	Щ	3n	ELBI 100E L CORE	1
(m)					SYMBOL	SOIL			E TYPE	E NO	TEST	DEPTH (ft)
ОЕРТН (m)	PLASTIC	M.C.	ı	Liquid —	SOIL S	DESCRIPT	ION	WELI	SAMPLI	SAMPLE	COMMENTS	DEPT
0.0	20	40 6	0 <u>8</u>	0		SILT — sandy, low to medium to wet, brownish grey	plastic, moist		1			0.0
- 1.0						to wet, brownian grey				D1		-
- 1.0						GRAVEL - loose, some sand, t	trace of silt,				Gravel = 62%	- 5.0
- 2.0	•					- moist at 1.8m				D2	Sand = 33% Fines = 5%	-
-						wet at 2.3mfrozen at 2.75m						-
- 3.0						- 11026H UL 2.75HI						- 10.0 -
- 4.0						End of Borehole at 4.0m				D3		<u></u>
-						End of Borendie dt 4.0m						- - 15.0
- 5.0												-
- 6.0												-
-												- 20.0 -
- 7.0												-
-												25.0
- 8.0												-
- 9.0												- 30.0
- 100												-
- 10.0 -												-
- 11.0											· ·	- 35.0 -
-												-
	<u> </u>	HI	RT .	ΔCF	?A	Limited	LOGGED BY: JLA		1		COMPLETION DEPTH: 4.0 m	1
						E, N.W.T.	REVIEWED BY:		_		COMPLETE: 93/09/29	1 -4 4
					- 111	<u>~, ^, </u>	Fig. No:				rage	1 of 1

PROJE	ECT: FORT	GOOD HO	PE AIRPOR	<u> </u>	METHOD: TRACK MOUNTED AUGER			В	OREHOLE No: 14	
CLIEN	T: DEPAR	tment of	TRANSPOR	TAT10				_	roject No: YX00354	
		MOBILE AUG		RESE			 	_	LEVATION: 0.000 (m)	_
SAMP	LE TYPE	DISTU	RBED	\angle	NO RECOVERY SPT	CASING_	 Щ	SHE	LBY TUBE CORE	1
DEPTH (m)	PLASTIC	M.C.	LIQUID	SOIL SYMBOL	SOIL DESCRIPTION		SAMPLE TYPE	SAMPLE NO	TEST COMMENTS	DEPTH (ft)
0.0	20	40 60	80		SILT top 250mm high in organics with		_			0.0
- 1.0 - 2.0					SILT — top 250mm high in organics with black silty sand — brownish grey silt at 0.25m — soft, low plastic, moist SAND AND GRAVEL — fine grained sand to coarse grained gravel, trace to little silt, wet, brown — saturated at 1.2m (hole flooded)					- 5.0
- 3.0					 hard drilling at 2.75m suspect frozen 					- - 10.0 -
- 4.0					End of Borehole at 4.0m					- - 15.0 -
- 6.0 -						٠				- 20.0
- 7.0 - - 8.0										- 25.0 -
- - 9.0 -									·	- 30.0
- 10.0 - - 11.0										- 35.0
-		НВ	T AGF	RA	Limited LOGGED BY:				COMPLETION DEPTH: 4.0 m	-
					E, N.W.T. REVIEWED B				COMPLETE: 93/09/29	1 -6 1

PROJE	CT: FORT	GOOD HOP	E AIRPOR	1	METHOD: TRACK N	OUNTED AUGER			_	BOREHOLE No: 15	
		MENT OF T							_	Project No: YX00354	
		OBILE AUG		RESE	ARCH LOCATION: STATION	N 0+735 A-CASING		П		ELBY TUBE CORE	
DEPTH (m)	PLASTIC	DISTUR	LIQUÍD.	SOIL SYMBOL	SOII DESCRIF		WELL INSTALLATION	YPE	SAMPLE NO	TEST	DEPTH (ft)
0.0	20	40 50	80		SAND AND SILT - loose, tro	ce of aravel.		Н			0.0
- 1.0	•				fine grained to medi reddish brown — damp, brownish gre GRAVEL — medium dense, s rounded, moist, brow	um grained, y at 0.6m andy, little silt,				Gravel = 7% Sand = 50% Fines = 43%	- - - - 5.0
- 2.0					– wet at 2.0m – frozen at 2.1m				D3		- -
- 3.0					•						- 10.0 -
- 4.0 -					End of Borehole at 4.0m						- - - 15.0
- 5.0 -											- - -
- 6.0 -											- 20.0
- 7.0 -											- - 25.0
- 8.0											-
- 9.0 -											- 30.0
- 10.0											75.0
- 11.0					,						- 35.0 -
		HB'	T AGE	?A	Limited	LOGGED BY: JLA				COMPLETION DEPTH: 4.0 m	
					FE, N.W.T.	REVIEWED BY: Fig. No:				COMPLETE: 93/09/30	1 of 1
1					-, - · · · · - ·	IIIU, IIU,				rude	I UI I

PROJE	ECT: FORT	GOOD HO	PE AIRPOR	<u> </u>	METHOD: TRACK MOU	NTED AUGER		BO	DREHOLE No: 16	
			TRANSPOR					_	oject No: YX00354	
			GERS AND	RESE				_	EVATION: 0.000 (m)	
SAMP	LE TYPE	DISTU	IRBED		NO RECOVERY SPT	= A-CASING	<u> </u>	SHE	LBY TUBE TORE	T
ОЕРТН (m)	PLASTIC	M.C.	LIQUID	SOIL SYMBOL	SOIL DESCRIPTI	ON WELL	SAMPLE TYPE	SAMPLE NO	TEST COMMENTS	DEPTH (ft)
0	20	40 50	80	X	5200111111		€ \$	S		
0.0	20	40 60	ou .		SAND — loose, trace of silt, dry reddish brown	to damp,				0.0
- 1.0					 fine grained to medium damp, brown with greyis silt seams at 0.6m frozen at 1.1m 	grained sand h white silt		D1		- 5.0
- 2.0	•				SAND AND GRAVEL — fine grains fine grained gravel, trace frozen	ed sand to e of silt,		D3		-
- 3.0					- ice crystals at 3.1m					10.0
- 4.0					End of Borehole at 4.0m			D4		- 15.0
- 5.0										-
- 6.0 -										- - 20.0
- 7.0 -										- 25.0
- 8.0										-
9.0										- 30.0
- 10.0										- -
- 11.0										- 35.0 - -
	; ;	ПП	የጥ ለሮፔ	λ. Σ	Limited	LOGGED BY: JLA			COMPLETION DEPTH: 4.0 m	
						REVIEWED BY:			COMPLETE: 93/09/30	
		I EL	TOWK	1111	E, N.W.T.	Fig. No:			l Page	1 of 1

		RT GOOD					METHOD: TRACK MC	OUNTED AUGER			_	BOREHOLE No: 17	
		RTMENT C					DATUM:	0.005			_	Project No: YX00354	
	ractor LE TYP	: MOBILE A	AUGE		RESE	arch No recover	LOCATION: STATION		ASING		_	ELEVATION: 0.000 (m) IELBY TUBE CORE	
DEPTH (m)	PLASTI	C M.C.		LIQUID	SOIL SYMBOL	NO RECOVER	SOIL DESCRIP	_		INSTALLATION SAMPLE TYPE	S S S	TEST	DEPTH (ft)
0.0	20	40	60	80		GRAVEL AN	ID SAND - loose, o	oarse grained	1	$\overline{\cdot}$	 		0.0
- 1.0	•					- tra	vel up to 50mm, r ce of silt, dry to d				D1	Gravel = 57% Sand = 40% Fines = 3%	- 5.0
- 2.0	•						mp at 1.5m GRAVEL – trace o	f silt			D2	Gravel = 37% Sand = 60% Fines = 3%	-
- 3.0							t at 2.6m zen at 2.75m						- 10.0
- 4.0 -						End of Bo	rehole at 4.0m				D3		- - - 15.0
- 5.0 -													- - -
- 6.0 -									,				- 20.0
- 7.0													- - - 25.0
- 8.0												`	- - -
- 9.0 -													- 30.0
- 10.0 -													- 35.0
- 11.0 L													
	_ ; ;	H	IB1	' AGF	RA	Limite	d	LOGGED BY:				COMPLETION DEPTH: 4.0 m	
						E, N.W		REVIEWED B	ST:			COMPLETE: 93/09/30 Page	1 of 1

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PROJE	CT: FOR	F GOOD HO	PE AIRPOR	RT.	METHOD: TRACK MOUNTED AUG	GER	BOREHOLE No: 18
		TMENT OF					Project No: YX00354
		MOBILE AU		RESE			ELEVATION: 0.000 (m)
SAMPI	LE TYPE	DISTU	RBED	Ľ	NO RECOVERY SPT	A_CASING S	HELBY TUBE CORE
ОЕРТН (ш)	PLASTIC 1——— 20	M.C.	Liquid 	SOIL SYMBOL	SOIL DESCRIPTION	WELL INSTALLATION SAMPLE TYPE SAMPLE TYPE	TEST (#) HEAR
0.0	20				SILT - soft, very fine sand, low plastic, moist to wet, brown		0.0
- 1.0	•				SAND AND SILT — loose, brown — frozen at 0.8m	. D	Gravel = 0% Sand = 61% Fines = 39%
- 2.0					GRAVEL AND SAND — fine grained to co grained sand, fine grained grave damp, brown		
- 3.0					- ice crystals noted at 2.6m		10.0
- 4.0					— sand content increased at 4.0m	03	- - - 15.0
- 5.0 -	•				- hard drilling at 4.9m SILT - clayey, brownish-grey - ice crystals noted in seams	. 04	
- 6 .0					GRAVEL AND SAND - ice crystals noted at 6.0m SILT - ice seams		20.0
- 7.0 -					End of Borehole at 7.0m		- 25.0
- 8.0							
- 9.0 -							- 30.0
- 10.0							35.0
- 11.0 -							
		HB	T AGE	RS		BY: JLA	COMPLETION DEPTH: 7.0 m
					FE, N.W.T. Fig. No:		COMPLETE: 93/10/01 Page 1 of 1

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PROJE	CT: FORT	GOOD HOP	PE AIRPOR	21		METHOD: TRACK MO	UNTED AUGER			B	OREHOLE No: 19	
		IMENT OF T				DATUM:				_	roject No: YX00354	
		OBILE AUG		RESE		LOCATION: STATION					LEVATION: 0.000 (m)	
SAMP	LE TYPE	DISTUR	RBED	<u> </u>	NO RECOVER	Y SPT	A-CASING		Щ	SHE	LBY TUBE CORE	
(m)				SYMBOL		SOIL		LATION		LE NO	TEST	DEPTH (ft)
ОЕРТН (m)	PLASTIC	M.C.		SOIL S		DESCRIPT	ION	WE	SAMPLE	SAMPLE	COMMENTS	DEPT
0.0	20	40 60	80			ose, little gravel, tr ty brown	ace of silt			D1		0.0
- 1.0]	to o	GRAVEL — loose, fi coarse grained sand e silt, brown						5.0
- 2.0												- -
- 3.0						t at 2.75m zen at 2.9m		ļ.		D2		- 10.0 -
- 4.0												- - - 15.0
- 5.0 -										D3		- - -
- 6.0					SAND - lit to (tle to some gravel, coarse grained san	fine grained d, frozen, wet			D4		20.0
- 7.0					End of Bor	rehole at 7.0m						- - - 25.0
- 8.0 -												- - -
- 9.0												- 30.0
- 10.0												- -
- 11.0												- 35.0 - -
		TTT-	m + ~=	<u></u>	T		LOGGED BY: JLA				COMPLETION DEPTH: 7.0 m	ŀ
					Limite		REVIEWED BY:				COMPLETE: 93/10/01	
		YEL	LOWK	.NI	E, N.W.	Т.	Fig. No:					1 of 1

PROJE	CT: FORT	GOOD HO	OPE AIRPOR	ī	METHOD: TRACK MOUNTED AUGER		BOREHOLE No: 20
		_	TRANSPOR	_			Project No: YX00354
			IGERS AND	RESE			ELEVATION: 0.000 (m)
SAMP	LE TYPE	DIST	URBED		NO RECOVERY SPT A-C	ASING US	HELBY TUBE CORE
DEРТН (m)	PLASTIC	M.C.	LQUID	SOIL SYMBOL	SOIL DESCRIPTION	WELL INSTALLATION SAMPLE TYPE SAMPLE NO	TEST (1) ELG
0.0	20	<u>40</u> 50	D 80		SAND AND GRAVEL — fine grained sand to	- - -	0.0
- 1.0					coarse gained gravel, trace of silt rusty brown		-
- 2.0 -							- 5.0
- 3.0 -							10.0
- 4.0 - - 5.0					— suspect frozen at 4.6m	D1	- - 15.0
- - 6.0 -							- - - 20.0
- 7.0 -					End of Borehole at 7.0m		- 25.0
- 8.0							. [
- 9.0							- 30.0
- 10.0 - - 11.0							- 35.0
-							
		H	BT AGE	RΣ	Limited LOGGED BY:		COMPLETION DEPTH: 7.0 m
					E, N.W.T.	1.	Page 1 of 1

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PROJ	ECT: FORT G	OOD HOPE A	NRPORT		METHOD: TRACK MOUNTED AUGER			В	OREHOLE No: 21	
	IT: DEPARTME		_		DATUM:			_	roject No: YX00354	
	RACTOR: MOI			_			_		LEVATION: 0.000 (m)	
SAMP	PLE TYPE	DISTURBED	1	<u> </u>	NO RECOVERY SPT A-CASING		Щ	SHE	LBY TUBE CORE	_
DEРТН (m)	PLASTIC 1-	M.C.	LIQUID	SOIL SYMBOL	SOIL DESCRIPTION	WELL INSTALLATION	SAMPLE TYPE	SAMPLE NO	TEST	DEPTH (ft)
0.0	20 4	10 <u>60 8</u>	30	9	CAND AND CRAVEL - fine grained sand to					0.0
- 1.0				3	SAND AND GRAVEL — fine grained sand to coarse gained grovel, trace of silt dry, rusty brown . — dark brown at 0.5m					- 5.0
- 2.0 -										- 5.0 -
- 3.0		•			– damp at 3.7m					- 10.0 L
- 4. 0 - - 5.0				S	GAND — trace to little gravel, fine grained to coarse grained sand, trace of silt, moist, brown			D1		- - 15.0 - -
- 6.0					- suspected frozen at 6.4m					- 20.0 -
- 7.0 -				Ē	ind of Borehole at 7.0m	-		D2 \		- - - 25.0
- 8.0									·	- - -
- 9.0										- 30.0 -
- 10.0										- - 35.0
11.0					1				Toolin Time	- -
		HBT	AGR <i>i</i>	A L	imited LOGGED BY: JLA REVIEWED BY:				COMPLETION DEPTH: 7.0 m COMPLETE: 93/10/01	
		YELLO)WKN	IIFE	E, N.W.T. Fig. No:		_		Page 1	of 1

PROJE	ECT: FORT G	OCD HOP	E AIRPOR	<u> </u>	METHOD: TRACK MOUNTED AUGER		BOREHOLE No: 22	
	T: DEPARTME			_			Project No: YX00354	
	RACTOR: MOI			RESE		ASING T	ELEVATION: 0.000 (m) SHELBY TUBE	
SAMP	LE TYPE	DISTURE	SEU	<u> </u>	NO RECOVERY SPT == A-0	ASING III	SHELET TOBECOKE	
ОЕРТН (m)	PLASTIC	M.C.	LIQUID	SOIL SYMBOL	SOIL DESCRIPTION	WELL INSTALLATION SAMPLE TYPE	TEST (1) HLG30	
	20	10 _ 50	80	S				
0.0					SILT — soft, trace to little sand, low to medium plastic, maist, beige		0.0	
- 1.0					SAND — fine grained to coarse grained sar moist, frozen, brawn — ice lenses noted at 1.2m	nd	D1	
- 2.0	•				- some gravel and trace to little silt at 1.5m		Gravel = 14% 03 Sand = 78%	
- 3.0				ı			Fines = 8%	
4.0							D4	
- 5.0							- 15.0 - 15.0	
- 6.0							- 20.0	
- 7.0					End of Borehole at 7.0m		06	
- 8.0							25.0	
- 9.0							- - 30.0	
- 10.0							-	
- 11.0							35.0	
					LITTILLE CO. BEVIEWED B	LOGGED BY: JLA COMPLETION DEPTH: 7.0 m REVIEWED BY: COMPLETE: 93/10/01		
	YELLOWKNII			NII	Fig. No:		Page 1 of 1	

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PROJE	CT: FORT	GOCD H	OPE AIRPO	R <u>T</u>	METHOD: TRACK MOUN	TED AUGER	_	OREHOLE No: 23	_
			TRANSPOR				_	roject No: YX00354	
			ugers and	_		840 5.5m SOUTH OF CL	_	LEVATION: 0.000 (m)	
SAMP	LE TYPE	DIS	TURBED		NO RECOVERY SPT	A-CASING []	SHI	ELBY TUBE CORE	_
ОЕРТН (т)	PLASTIC	M.C.	LIQ UID	SOIL SYMBOL	SOIL DESCRIPTION	WELL NETALATION SAMPLE TYPE	SAMPLE NO	TEST COMMENTS	DEPTH (ft)
0.0	20	40 6	50 80	├	SAND AND GRAVEL - coarse gra	pined gravel.	_		0.0
- 1.0					fine to coarse grained so to little silt, cobbles near surface, dry, brown	and, trace			-
- 2.0							D1		- 5.0 -
- 3.0					SAND — some gravel, trace of s brown	silt, damp,	D2		10.0
- 4.0							UZ		- - 15.0
- 5.0							D3		- - -
- 6.0									- 20.0
7.0					- trace of gravel at 6.7m		L4		- 25.0
- 8.0					- little silt at 7.6m				25.0
9.0					End of Borehole at 8.5m				- 30.0
10.0									- - -
11.0									35.0
		TT	יים א מיים		T::1 1	LOGGED BY: JLA		COMPLETION DEPTH: 8.5 m	<u> </u>
					Lillited	REVIEWED BY:		COMPLETE: 93/10/01	
		YE	LLOWK	NIF		Fig. No:			1 of 1

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METHOD: TRACK MOUNTED AUGER BOREHOLE No: 24 PROJECT: FORT GOOD HOPE AIRPORT DATUM: Project No: YX00354 CLIENT: DEPARTMENT OF TRANSPORTATION CONTRACTOR: MOBILE AUGERS AND RESEARCH LOCATION: STATION 3+840 60m North of CL ELEVATION: 0.000 (m) **⊠** SPT SAMPLE TYPE DISTURBED NO RECOVERY A-CASING SHELBY TUBE CORE WELL INSTALLATION SAMPLE TYPE SYMBOL € $\overline{\mathbb{E}}$ SOIL. TEST SAMPLE DEPTH (DEPTH (DESCRIPTION COMMENTS PLASTIC M.C. LIQUID SOIL 80 20 0.0 0.0 GRAVEL AND SAND - medium grained sand (gravel sizes to 20mm), trace of silt, dry, rusty brown 1.0 fine grained to coarse grain gravel sizes, damp, dark brown at 1.1m Gravel = 54% 5.0 Sand = 43%D1 Fines = 3%2.0 SAND - little gravel (fine grained), fine 3.0 10.0 to coarse grained sand, trace of D2 silt, damp to moist, brown 4.0 - frozen at 4.0m 15.0 5.0 6.0 20.0 some coarse grained gravel sizes at 6.1m. D3 - saturated at 6.1m 7.0 25.0 8.0 9.0 30.0 10.0 End of Borehole at 10.1m 35.0 11.0 LOGGED BY: JLA COMPLETION DEPTH: 10.1 m HBT AGRA Limited REVIEWED BY: COMPLETE: 93/10/01 YELLOWKNIFE, N.W.T. Fig. No: Page 1 of 1

PROJE	CT: FORT	GOCD HOPE	E AIRPOR	τ		METHOD: TRACK N	AOUNTED AUGER		_	. [BOREHOLE No: 25	
		MENT OF TR				DATUM:				$\overline{}$	Project No: YX00354	
		<u>iobile auge</u>		RESE			N 3+600 60m South		г		ELEVATION: 0.000 (m)	
SAMPL	LE TYPE	DISTURE	BED -	\mathcal{L}	NO RECOVERY	∑ SPT	A-CASINO	-	Т	∐ SH	ELBY TUBE CORE	
DEРТН (m)	PLASTIC	M.C. 40 60	LIQUID 80	SOIL SYMBOL		SOII DESCRIF		WELL	SAMPLE TYPE	SAMPLE NO	TEST COMMENTS	DEPTH (ft)
0.0			CU		medi dry,	um grained san rusty brown						0.0
- 1.0 -					— dark	. brown at 0.8m				D1	Gravel = 50% Sand = 46% Fines = 4%	- 5.0
2.0										וט		
3.0					sand	grained gravel, , trace of silt, d p, brown	fine grained lamp, brown at			D2	Gravel = 52% Sand = 45% Fines = 3%	- 10.0 - -
					– susp – little	ected frazen at gravel at 4.3m	4.3m					- 15.
· 5.0 ·						•				D3		- -
· 6.0 ·					- som	e gravel at 6.1r	n			D4		- 20.
· 7.0 ·												- 25.
8.0										D5		-
9.0												- 30.
10.0					End of Bore	hole at 10.1m		_		D6		- - - 3 5.
11.0												-
		HBT	AGF	RA	Limited		LOGGED BY: JLA				COMPLETION DEPTH: 10	.1 m
					E, N.W.		REVIEWED BY: Fig. No:				COMPLETE: 93/10/02	age 1 of

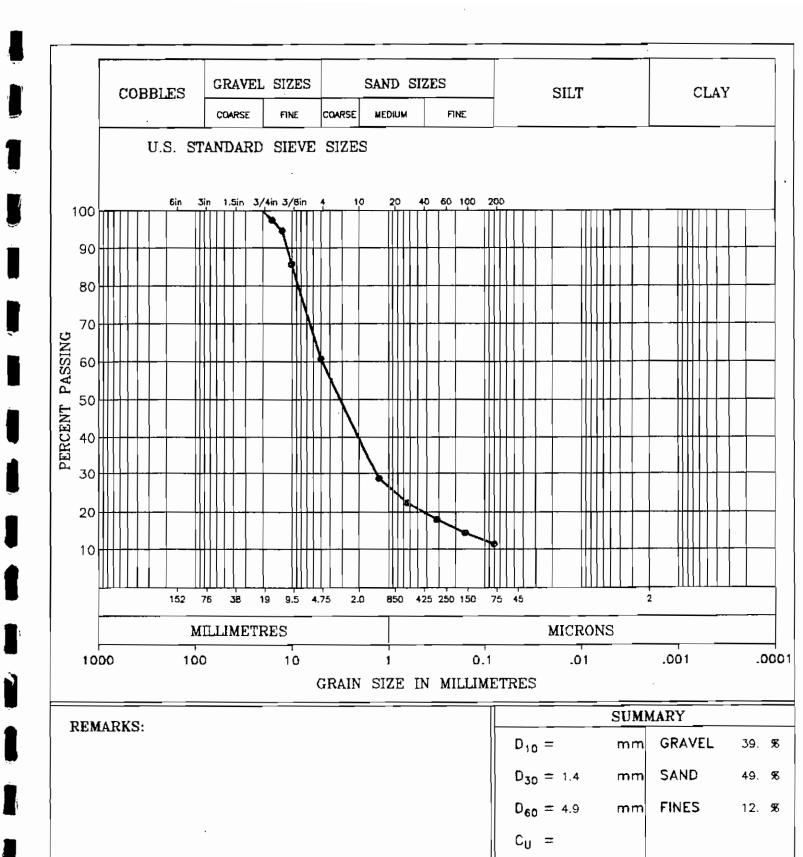
BOREHOLE No: 26 PROJECT: FORT GOOD HOPE AIRPORT METHOD: TRACK MOUNTED AUGER CLIENT: DEPARTMENT OF TRANSPORTATION DATUM: Project No: YX00354 LOCATION: STATION 3+380 60m South of CL CONTRACTOR: MOBILE AUGERS AND RESEARCH ELEVATION: 0.000 (m) SHELBY TUBE A-CASING CORE SAMPLE TYPE DISTURBED NO RECOVERY SAMPLE TYPE SYMBOL 용 Œ **SOIL** DEPTH (ft) TEST SAMPLE DEPTH DESCRIPTION PLASTIC LIQUID COMMENTS SOIL 20 40 60 80 0.0 0.0 SAND AND GRAVEL - coarse grained gravel fine grained sand, trace of silt, dry, rusty brown - dark brown at 0.8m 1.0 5.0 2.0 GRAVEL AND SAND - fine to coarse grained Gravel = 49% 3.0 10.0 sand, trace of silt, wet, brown Sand = 47%- suspected frozen at 3.0m D1 Fines = 4%4.0 15.0 5.0 D2 6.0 20.0 little gravel at 4.3m D3 7.0 25.0 8.0 D4 9.0 30.0 10.0 End of Borehole at 10.1m 35.0 11.0 LOGGED BY: JLA COMPLETION DEPTH: 8.5 m HBT AGRA Limited COMPLETE: 93/10/02 REVIEWED BY: YELLOWKNIFE, N.W.T. Fig. No: Page 1 of 1

PROJI	ECT: FOR	T GOOD H	IOPE	AIRPOR	श	METHOD: TRACK M	OUNTED AUGER			В	OREHOLE No: 27	
	T: DEPAR						7.000	^:		-	roject No: YX00354	
	RACTOR:				RESE		3+260 60m North of	CL	П		LEVATION: 0.000 (m) LBY TUBE CORE	_
SAMP	LE TYPE	DIS	TURB	EU .		NO RECOVERY X SPT	A-CASING		Щ	Jane	TBT TOBE	
ОЕРТН (т)	PLASTIC	M.C.		LIQUID	IL SYMBOL	SOIL DESCRIP		WELL INSTALLATION	SAMPLE TYPE	SAMPLE NO	TEST COMMENTS	ОЕРТН (ft)
8	<u></u>	40			SOIL	DESCRI	11011	Z	SAI	3		
0.0	20	40 1	50	80		SAND AND GRAVEL - coarse fine grained sand, tro dry, rusty brown				D1	-	0.0
- 1.0						– dark brown at 0.8m						- - - 5.0
- 2.0	•									D2		-
- 3.0						SAND — little to some grave silt, wet, brown — suspected frozen at						- 10.0 -
- 4.0										D3		15.0
- 5 .0					-	— trace to little gravel	at 4.6m					-
- 6.0						— trace of gravel at 6.	1 m			D4		- - 20.0
- 7.0 -						SiLT — clayey, medium plast	ic, grey					- 25.0
- 8.0											-	-
9.0						End of Borehole at 8.5m						- - 30.0
- 10.0												- - -
- 11.0												35.0
							Lagery St. T.				Topy of the second seco	-
						Limited	LOGGED BY: JLA REVIEWED BY:				COMPLETION DEPTH: 8.5 m COMPLETE: 93/10/02	
		YE	LL	OWK	NIF	E, N.W.T.	Fig. No:					1 of 1

	CT: FORT C				METHOD: TRACK MOU	NTED AUGER			_	OREHOLE No: 28	
	T: DEPARTM						01		_	Project No: YX00354	
	RACTOR: MO LE TYPE	DISTUR		RESE	TARCH LOCATION: STATION 3- NO RECOVERY SPT	+160 60m South of A-CASING	CL	П		LEVATION: 0.000 (m) ELBY TUBE CORE	
SAMP	LE TIPE	UISTUR	IDEU		NO RECOVER SF			Щ] 311	TID TOBL CORL	-
ОЕРТН (m)				SYMBOL	SOIL		WELL Installation	E IYPE	CE NO	TEST	н (т
DEPT	PLASTIC +	M.C.	LIQUID	SOIL S	DESCRIPT	ION	NSTAL	SAMPLE	SAMPLE	COMMENTS	DEPTH
0.0	20	<u>40 50</u>	80	_	SILT — soft, clayey, low to med	fium		Н			0.0
[plastic, moist to wet, br	ownish		l			-
					grey — frozen at 0.6m						Ĺ
1.0				1							[
-				-	GRAVEL — fine grained gravel, grained sand, trace to I	some fine ittle silt					- 5.0
					wet, brown	icus siic					 -
2.0				1					D1		-
+			ļļ								<u> </u>
- 3.0											10.0
										0	- 10.0
<u> </u>	•			1					D2	Gravel = 77% Sand = 20%	-
- 4.0				-						Fines = 3%	-
											ŀ
						1					- 15.0
- 5.0				1							[
-			ļļļ						· D3		-
											}
6.0				1	– hard drilling at 6.1m						20.0
-				-	, , , , , , , , , , , , , , , , , , ,						-
7.0					,						<u> </u>
7.0											
-			-		CAND Little to game fine area	inad assual	ļ				- 25.0
- 8.0		.ļļ]	SAND — little to some fine grain coarse grained sand, tro						-
					wet, brown	·					-
<u> </u>			†								-
- 9.0				-							- 30.0
					SILT — low to medium plasticity	y, frozen.					_ 30.0
					brown-grey						-
10.0				1	End of Doroholo at 10.1-						-
-					End of Borehole at 10.1m						
											- 35.0
- 11.0											
-											
											<u>-</u>
		HB	T AGI	RA	Limited	LOGGED BY: JLA				COMPLETION DEPTH: 10.1 m	1
	YELLOWKNIFE, N.W					REVIEWED BY: Fig. No:				COMPLETE: 93/10/02	1 of 1

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PROJECT: FORT GOOD HOPE AIRPORT					METHOD: TRACK N	MOUNTED AUGER				_	OREHOLE No: 29		
			F TRANSPO			DATUM:	. 7,070,400				_	roject No: YX00354	
			JUGERS ANI TURBED	RESI	ARCH NO RECOVERY		N 3+650 100m Nor		CL	П	_	LEVATION: 0.000 (m) LBY TUBE CORE	
DEPTH (m)	PLASTIC	M.C.	LIQUII	SOIL SYMBOL IN	INO RECOVERY	SOII DESCRIF	_ 	- I			SAMPLE NO	TEST COMMENTS	DEPTH (ft)
0.0	20	40 (60 80	+	SAND AND	GRAVEL - coarse	argined gravel	\dashv	_	-			0.0
- 1.0					fine dry,	grained sand, tr rusty brown wn at 0.8m	ace of silt,						- 5.0
- 2.0									l				- - -
- 3.0					silt,	tle to some grave wet, brown grained sand to							- 10.0 -
- 4.0					_	spected frazen at	4.3m						- 15.0
- 5.0 - - 6.0					End of Bore	ehole at 5.5m							-
7.0													- 20.0
- 8.0													- - 25.0 -
- 9.0													- 30.0
- 10.0													-
- 11.0													- 35.0 - -
		TŤ	ם א חכו	D.	T i ! ! .		LOGGED BY: JLA					COMPLETION DEPTH: 5.5 m	<u> </u>
	HBT AGRA Limited			U T	REVIEWED BY:				,	COMPLETE: 93/10/02			
	YELLOWKNIFE, N.W.				 , 1₹. ₹₹		Fig. No:					l Page	1 of 1



GRAIN SIZE DISTRIBUTION

PROJECT No: YX00354.200

 $c_c =$

DATE: 93.11.03

LOCATION: FORT GOOD HOPE AIRPORT

HOLE:

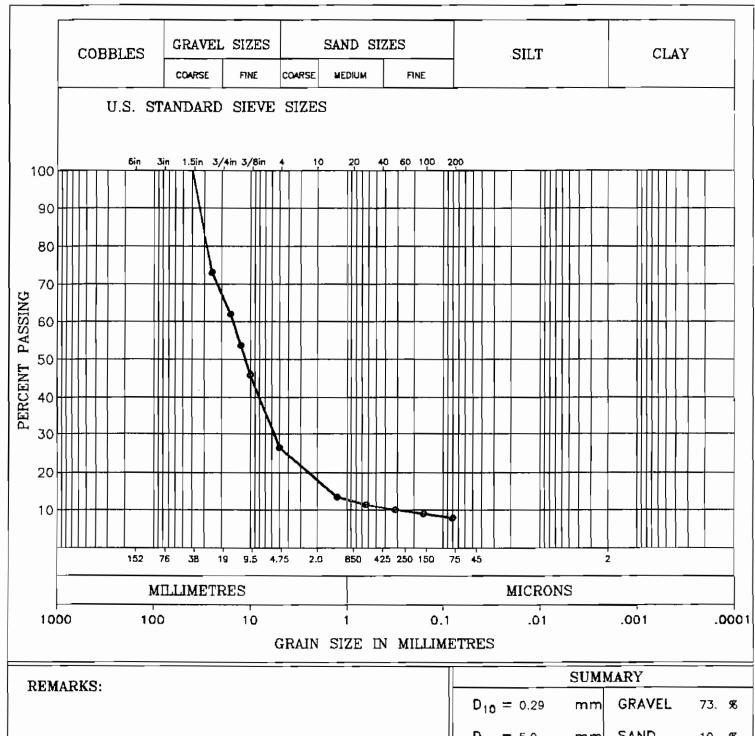
BH1

SAMPLE:

DEPTH:

3.35-3.96

TECHNICIAN: LR



REMARKS:				_	-
	$D_{10} = 0.29$	mm	GRAVEL	73. %	
	D ₃₀ = 5.9	mm	SAND	19. %	
	D ₆₀ = 15.	mm	FINES	8. %	
	C _U = 51.				
	C _C = 7.8				

GRAIN SIZE DISTRIBUTION

PROJECT No: YX00354.200

DATE: 93.11.03

LOCATION:

FORT GOOD HOPE AIRPORT

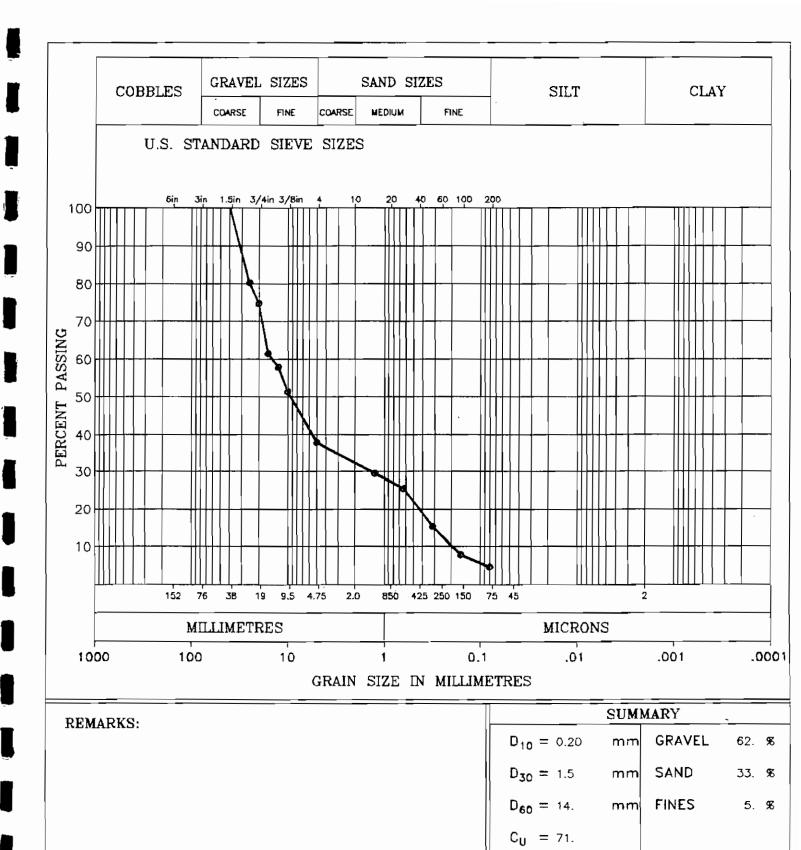
HOLE: BH.5

SAMPLE:

DEPTH:

1.83-2.13 m

TECHNICIAN: LR



GRAIN SIZE DISTRIBUTION

PROJECT No: YX00354.200

 $C_C = 0.72$

BH.13

DATE: 93.11.03

LOCATION: FORT GOOD HOPE AIRPORT

HOLE:

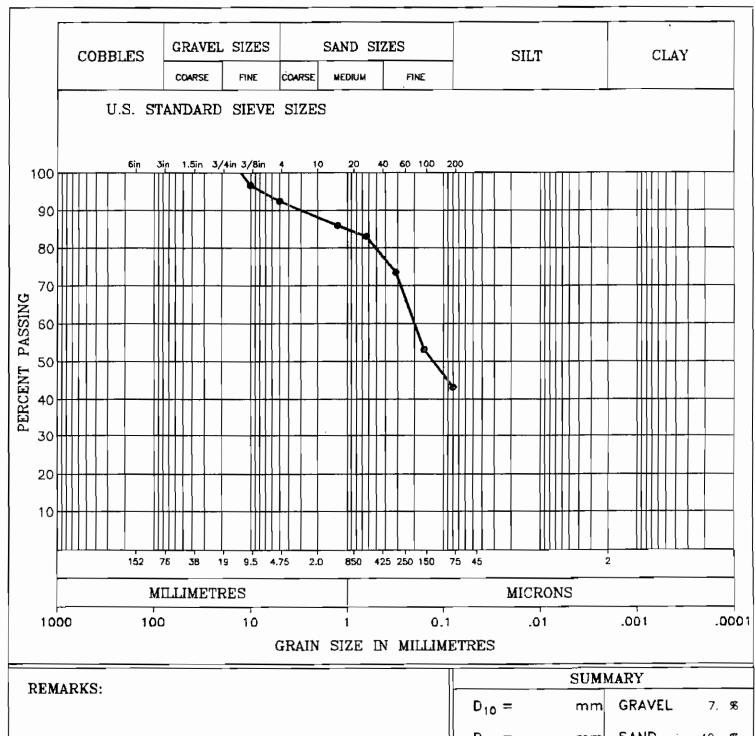
SAMPLE:

m

DEPTH:

1.52 - 2.13

TECHNICIAN: LR



D ₁₀ =	mm	GRAVEL	7. %	
D ₃₀ =	mm	SAND	49. %	
D ₆₀ = 0.21	mm	FINES	44. %	
C _U =				
C _C =				

GRAIN SIZE DISTRIBUTION

PROJECT No: YX00354.200

DATE: 93.11.03

LOCATION:

FORT GOOD HOPE AIRPORT

HOLE:

BH.15

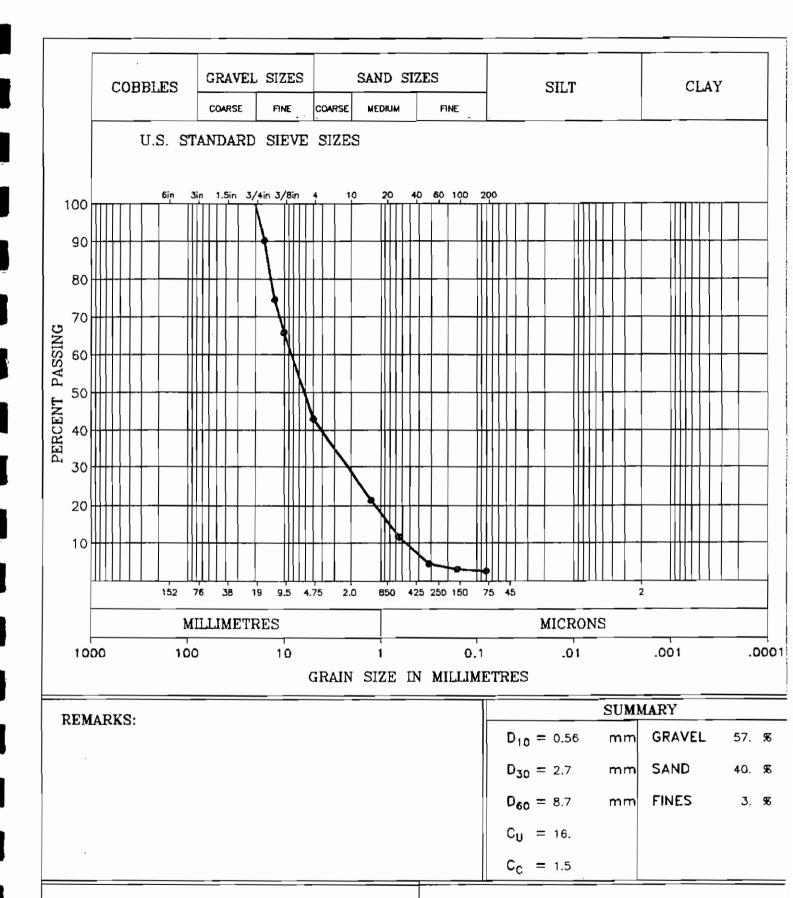
SAMPLE:

DEPTH:

0.91

m

TECHNICIAN: LR



GRAIN SIZE DISTRIBUTION

PROJECT No: YX00354.200

DATE: 93.11.03

LOCATION: FORT GOOD HOPE AIRPORT

HOLE:

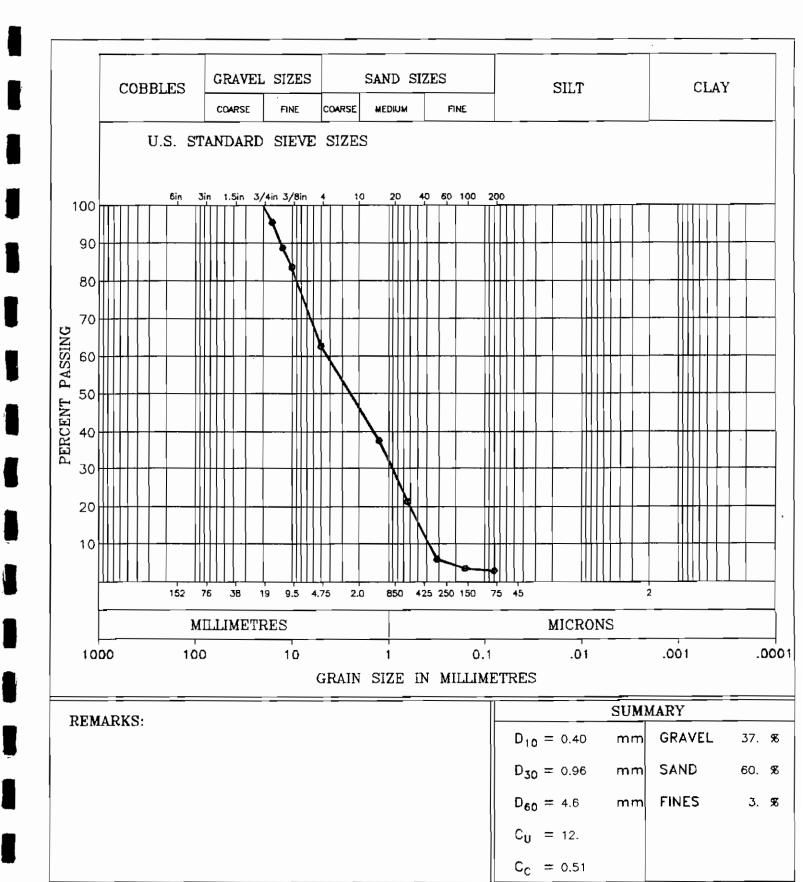
BH.17 SAMPLE:

DEPTH:

0.61 - 0.91

m

TECHNICIAN: LR



GRAIN SIZE DISTRIBUTION

PROJECT No: YX00354.200

DATE: 93.11.03

LOCATION: FORT GOOD HOPE AIRPORT

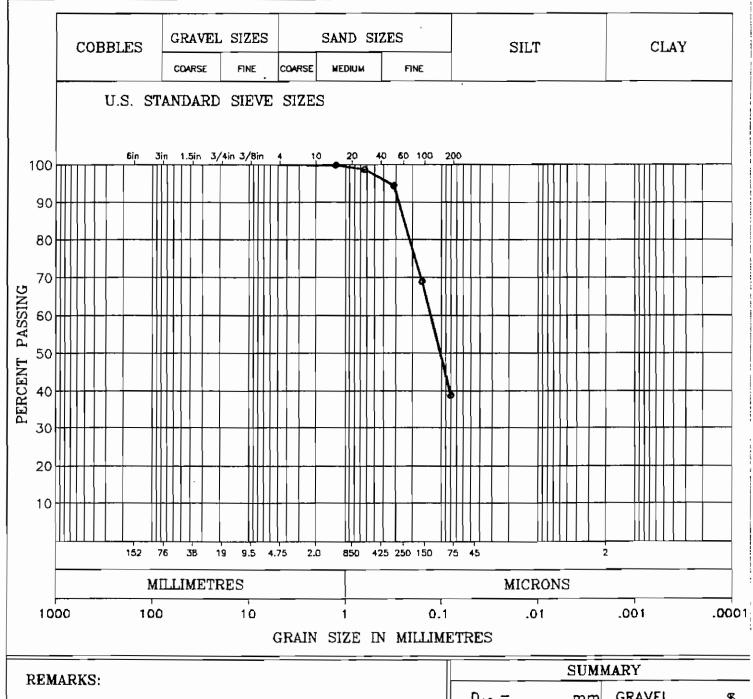
HOLE:

SAMPLE: BH.17

DEPTH:

1.83 - 2.44m

TECHNICIAN: LR



REMARKS:	SUMMARI						
NEMANO.	D ₁₀ =	mm	GRAVEL	95			
	D ₃₀ =	mm	SAND	61. %			
	$D_{60} = 0.14$	mm	FINES	39. %			
	C _U =						
	c _c =						

GRAIN SIZE DISTRIBUTION

PROJECT No: YX00354.200

DATE: 93.11.03

LOCATION: HOLE:

FORT GOOD HOPE AIRPORT

SAMPLE:

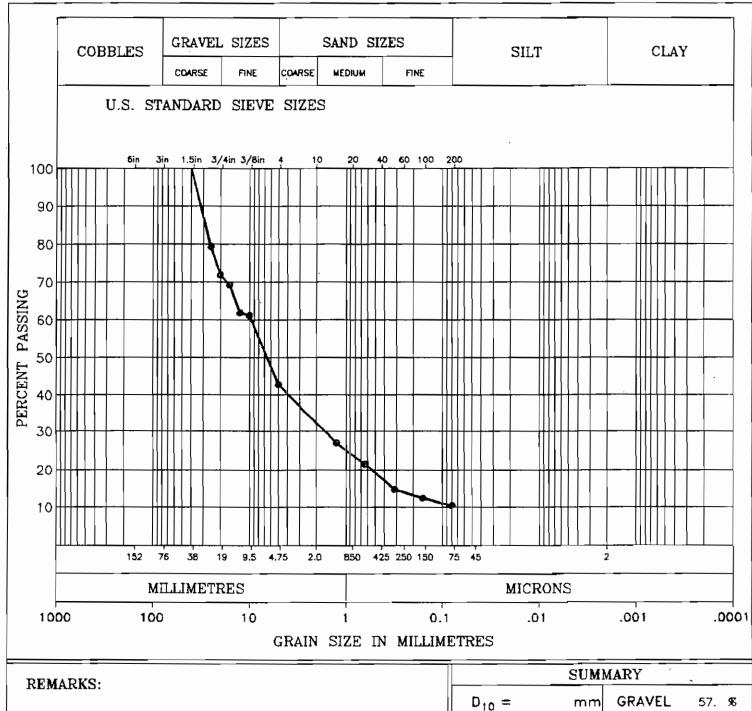
DEPTH:

0.91

BH.18

m

TECHNICIAN: LR



REMARKS:	SUMMARI					
WEIGHT TO THE TOTAL THE TOTAL TO THE TOTAL TOTAL TO THE T	D ₁₀ =	mm	GRAVEL	57. %		
	D ₃₀ = 2.0	mm	SAND	32. %		
	D ₆₀ = 9.7	mm	FINES	11. 95		
	c _u =					
	c _c =					

GRAIN SIZE DISTRIBUTION

PROJECT No: YX00354.200

DATE: 93.11.03

LOCATION:

FORT GOOD HOPE AIRPORT

HOLE:

m

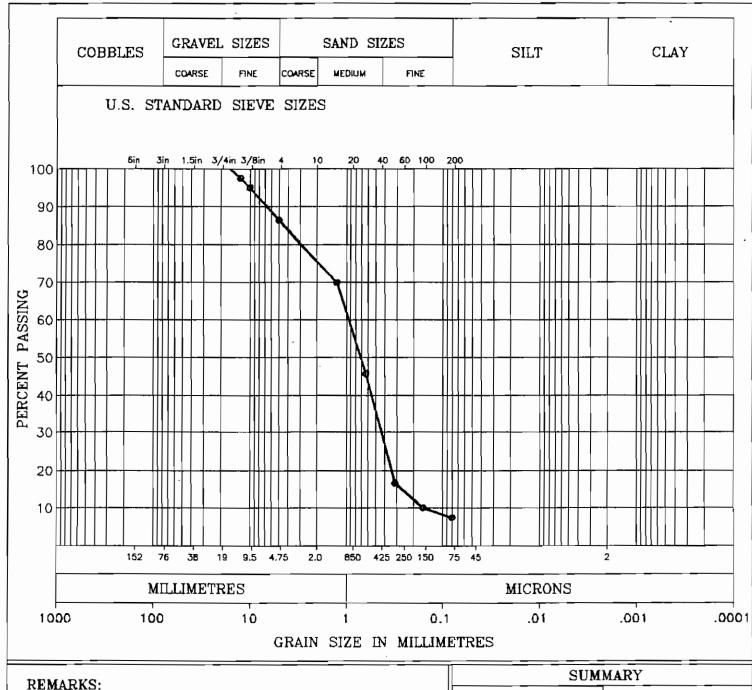
BH.19

SAMPLE:

DEPTH:

0.61

TECHNICIAN: LR



REMARKS:	SUMMAF								
MUNICIO.	$D_{10} = 0.16$	mm	GRAVEL	14. %					
	$D_{30} = 0.46$	mm	SAND	79. %					
	D ₆₀ = 0.99	mm	FINES	7. %					
	C _U = 6.3								
	$C_{C} = 1.3$								

GRAIN SIZE DISTRIBUTION

PROJECT No: YX00354.200

DATE: 93.11.03

LOCATION:

FORT GOOD HOPE AIRPORT

HOLE:

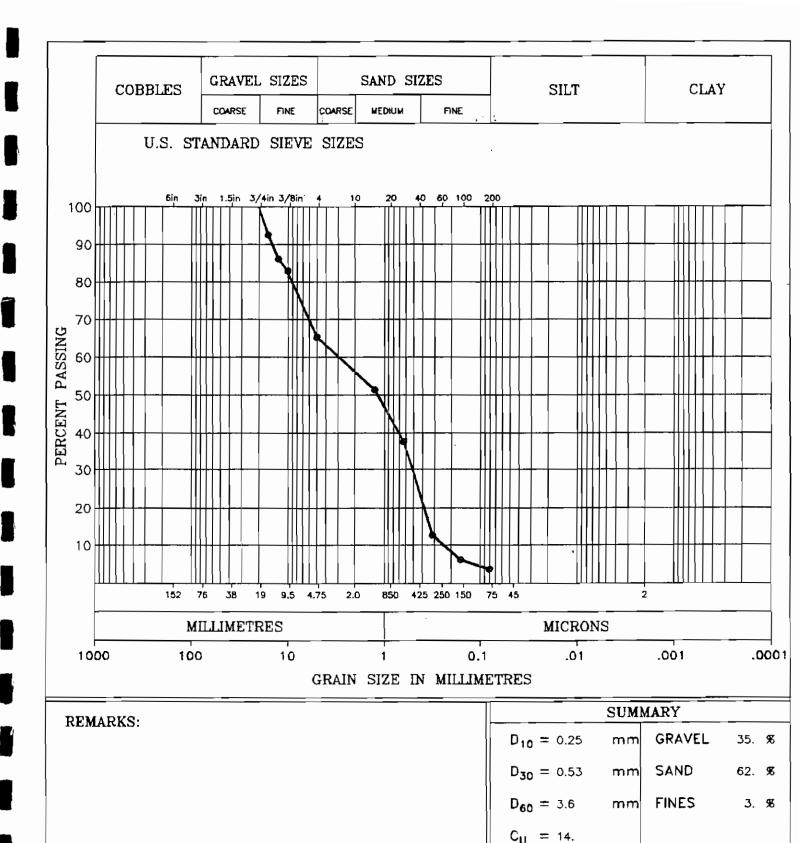
BH.22 SAMPLE:

m

DEPTH:

2.13-2.44

TECHNICIAN: LR



GRAIN SIZE DISTRIBUTION

PROJECT No: YX00354.200

 $C_{\rm C} = 0.32$

BH.23

DATE: 93.11.03

LOCATION: FORT GOOD HOPE AIRPORT

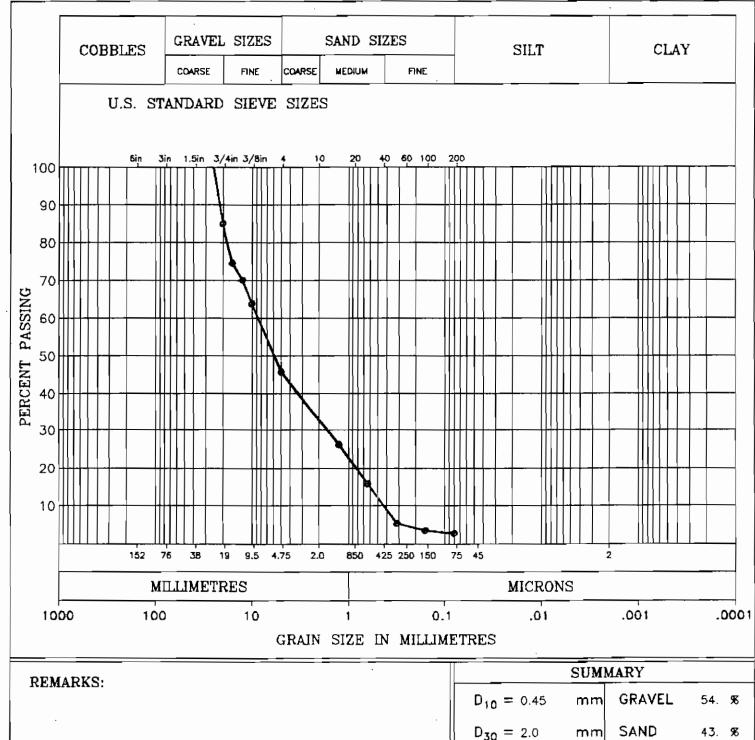
HOLE:

SAMPLE:

DEPTH:

7.01-10.06 m

TECHNICIAN: LR



$D_{10} = 0.45$	mm	GRAVEL	54. %
$D_{30} = 2.0$	mm	SAND	43. %
D ₆₀ = 8.9	mm	FINES	3. %
C _U = 20.			
$C_{C} = 0.97$			

GRAIN SIZE DISTRIBUTION

PROJECT No: YX00354.200

DATE: 93.11.03

LOCATION:

FORT GOOD HOPE AIRPORT

HOLE:

SAMPLE:

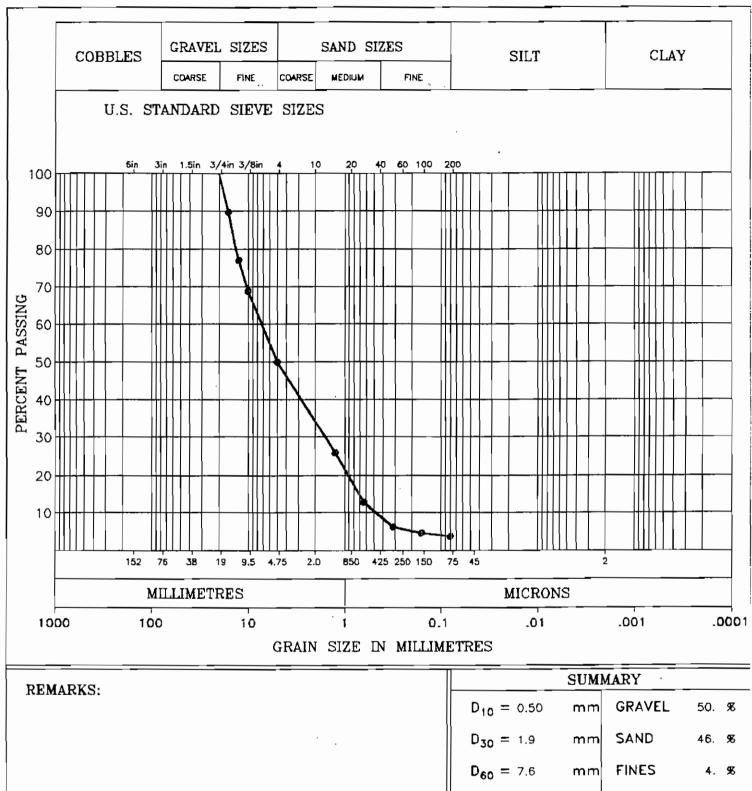
m

DEPTH:

1.52 - 2.44

BH.24

TECHNICIAN: LR



GRAIN SIZE DISTRIBUTION

 $D_{60} = 7.6$ mm FINES 4. % $C_U = 15.$ $C_C = 0.94$

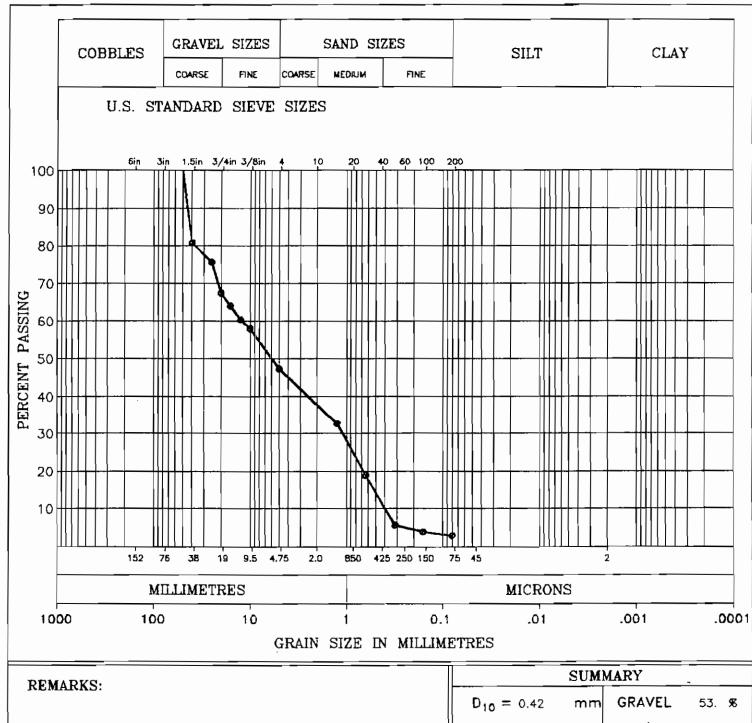
PROJECT No: YXD0354.200 DATE: 93.11.03

LOCATION: FORT GOOD HOPE AIRPORT

HOLE: BH.25 SAMPLE:

DEPTH: 1.52-2.44 m

TECHNICIAN: LR 166



REMARKS:	SUMMARY				
MARINED.	$D_{10} = 0.42$	mm	GRAVEL	53. %	
	D ₃₀ = 1.1	mm	SAND	44. %	
	D ₆₀ = 12.	mm	FINES	3. %	
	C _U = 29.				
	$C_{C} = 0.25$				

GRAIN SIZE DISTRIBUTION

PROJECT No: YX00354.200

DATE: 93.11.03

LOCATION:

FORT GOOD HOPE AIRPORT

HOLE:

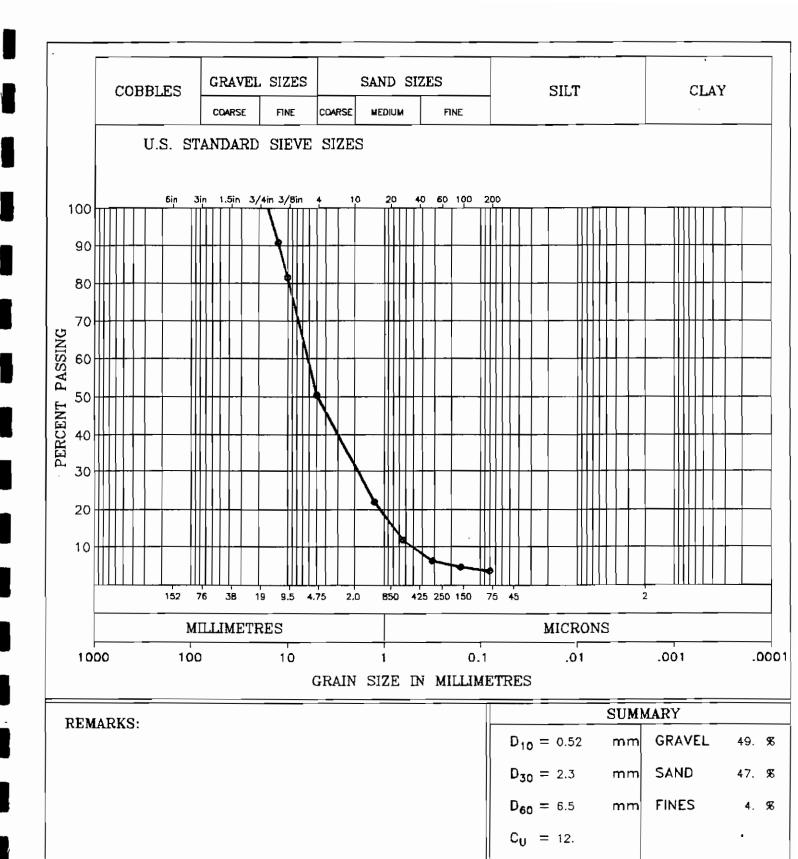
SAMPLE: BH.25

DEPTH:

3.05-3.96

m

TECHNICIAN: LR



GRAIN SIZE DISTRIBUTION

PROJECT No: YX00354.200

 $C_{C} = 1.6$

DATE: 93.11.03

LOCATION: FORT GOOD HOPE AIRPORT

HOLE:

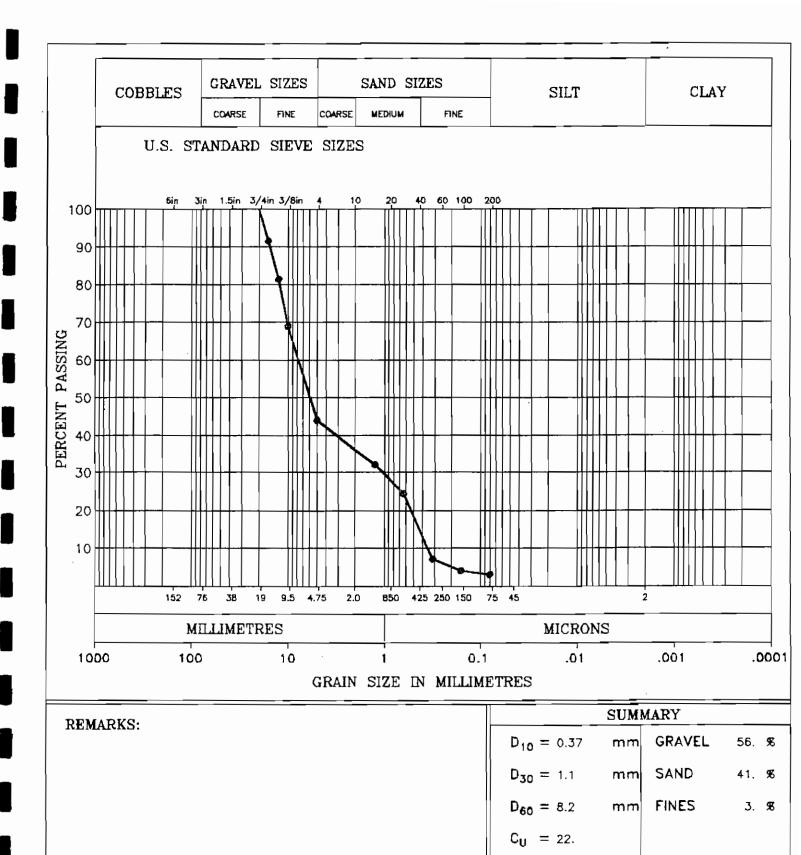
BH.26 SAMPLE:

m

DEPTH:

3.05-3.96

TECHNICIAN: LR



GRAIN SIZE DISTRIBUTION

PROJECT No: YXD0354.200

 $C_{\rm C} = 0.39$

DATE: 93.11.03

LOCATION: FORT GOOD HOPE AIRPORT

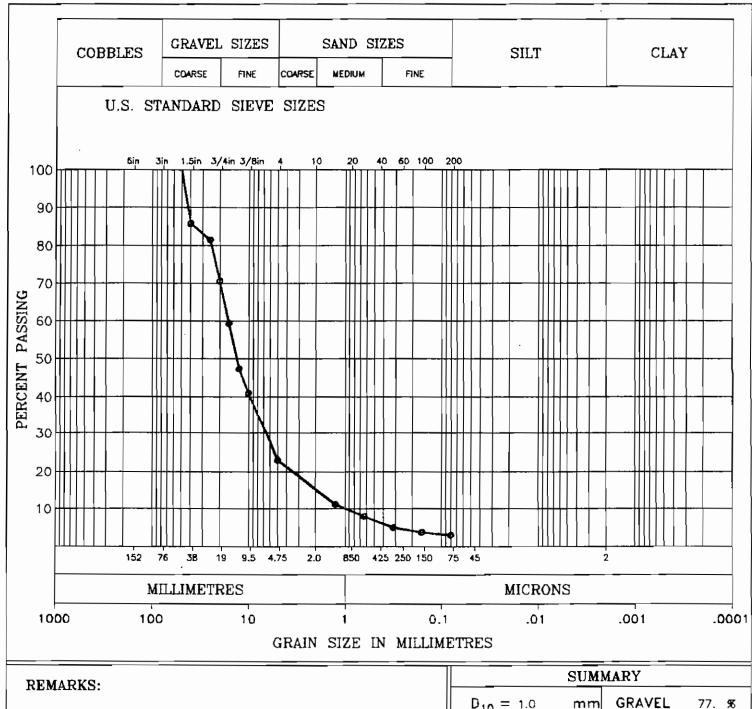
HOLE:

BH.27 SAMPLE:

DEPTH:

1.83 - 2.44

TECHNICIAN: LR



REMARKS:	S <u>UMMARY</u>				
	$D_{10} = 1.0$	mm	GRAVEL	77. %	
	D ₃₀ = 6.9	mm	SAND	20. %	
	D ₆₀ = 16.	mm	FINES	3. %	
	C _U = 16.				
	$C_{C} = 2.9$				

GRAIN SIZE DISTRIBUTION

PROJECT No: YX00354.200

DATE: 93.11.03

LOCATION:

FORT GOOD HOPE AIRPORT

m

HOLE:

BH.28 SAMPLE:

DEPTH:

3.35-3.96

TECHNICIAN: LR

APPENDIX C

Explanation of Terms and Symbols

Engineering & Environmental Services

EXPLANATION OF TERMS AND SYMBOLS

The terms and symbols used on the borehole logs to summarize the results of field investigation and subsequent laboratory testing are described in these pages.

It should be noted that materials, boundaries and conditions have been established only at the borehole locations at the time of investigation and are not necessarily representative of subsurface conditions elsewhere across the site.

TEST DATA

Data obtained during the field investigation and from laboratory testing are shown at the appropriate depth interval.

Abbreviations, graphic symbols, and relevant test method designations are as follows:

*C	Consolidation test	*ST	Swelling test
D_{R}	Relative density (formerly specific gravity)	TV	Torvane shear strength
Fines	Percentage by weight smaller than #200 sieve	vs	Vane shear strength (undisturbed-remolded)
k	Permeability coefficient	w	Natural water content (ASTM D 2216)
*MA	Mechanical grain size analysis and	\mathbf{w}_{l}	Liquid limit (ASTM D 423)
	hydrometer test	\mathbf{w}_{p}	Plastic limit (ASTM D 424)
N	Standard penetration test (CSA A119.1-60)	εţ	Unit strain at failure
N_d	Dynamic cone penetration test	Y	Unit weight of soil or rock
NP	Non plastic soil	γd	Dry unit weight of soil or rock
pp	Pocket penetrometer strength	ρ	Density of soil or rock
*q .	Triaxial compression test	$\rho_{\rm d}$	Dry density of soil or rock
\mathbf{q}_{u}	Unconfined compressive strength	, ,	•
*SB	Shearbox test	\rightarrow	seepage
SO ₄	Concentration of water-soluble sulphate	▼	observed water level

^{*}The results of these tests usually are reported separately

SOIL CLASSIFICATION AND DESCRIPTION

Soils are classified and described according to their engineering properties and behaviour.

The soil of each stratum is described using the Unified Soil Classification System¹ modified slightly so that an inorganic clay of "medium plasticity" is recognized.

The use of modifying adjectives may be employed to define the actual or estimated percentage range by weight of minor components. This is similar to a system developed by D.M. Burmister.²

The soil classification system is shown in greater detail on page 2.

SAMPLE TYPE — The type of sample is shown at the appropriate depth interval using the following abbreviations:

- A auger sample
- B block sample
- C rock core, or frozen soil core
- D drive sample/SPT sample
- P pitcher tube sample
- U tube sample (usually thin-walled)
- W wash or air return sample
- O other (see report text)
- indicates no sample recovery

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 [&]quot;Unified Soil Classification System", Technical Memorandum 3-357 prepared for Office, Chief of Engineering, by Waterways Experiment Station, Vicksburg, Mississippi, Corps of Engineers, U.S. Army. Vol. 1, March 1953.

American Society for Testing and Materials. Procedures for Testing Soils, "Suggested Methods of Testing for Identification of Soils", 4th Ed; pp 221-233, Dec. 1964.

MODIFIED UNIFIED CLASSIFICATION SYSTEM FOR SOILS

	MAJOR	DIVISION	GROUP SYMBOL	GRAPH SYMBOL	TYPICAL DESCRIPTION		ABORATORY ASSIFICATION CRITERIA	
	35_	CLEAN GRAVELS	GW	D: 50:5	WELL GRADED GRAVELS, LITTLE OR NO FINES	$C^{0} = \frac{D^{10}}{D^{00}} > 0$	$C_C = \frac{(D_{30})^2}{D_{10} \times D_{60}} = 1 \text{ to } 3$	
S HAN 200 SIEVE) GRAVELS THAN HAU COARSE HIS LARGER THAN		(LITTLE OR NO FINES)	GP		POORLY GRADED GRAVELS, AND GRAVELSAND MIXTURES, LITTLE OR NO FINES	ABC	NOT MEETING OVE REQUIREMENTS	
SOILS ER THAN 200	GRAVELS RE THAN HALF RAINS LARGER NO 4 SIEVI	DIRTY GRAVELS	GM		SILTY GRAVELS, GRAVEL-SAND-SILT MIXTURES	CONTENT OF FINES	ATTERBERG LIMITS BELOW "A" LINE OR P.I. LESS THAN 4	
	M O	DIRTY GRAVELS (WITH SOME FINES)	GC		CLAYEY GRAVELS, GRAVEL-SAND- CLAY MIXTURES	EXCEEDS 12%	ATTERBERG LIMITS ABOVE "A" LINE P.I MORE THAN 7	
COARSE-GRAINED	w z	CLEAN SANDS	sw		WELL GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES	$c_0 = \frac{D_{60}}{D_{10}} >$	$>6 C_C = \frac{(D_{30})^2}{D_{10} \times D_{60}} = 1 \text{ to } 3$	
COA THAN HALF	IDS HALF FINE HER THAN SIEVE	(LITTLE OR NO FINES)	SP		POORLY GRADED SANDS, LITTLE OR NO FINES		NOT MEETING OVE REQUIREMENTS	
(MORE TH	SANI SANI RE THAN AINS SMAI	MORE THAN HALE GRANIUS SANTIUR O 4 SIEVE O 5 SIEVE O 6 SIEVE O 7 SIEVE O 7 SIEVE O 8 SIEVE O 9 SIEVE	SM		SILTY SANDS, SAND-SILT MIXTURES	CONTENT OF FINES	ATTERBERG LIMITS BELOW "A" LINE P.I. LESS THAN 4	
	Σ Ö		sc		CLAYEY SANDS, SAND-CLAY MIXTURES	EXCEEDS 12%	ATTERBERG LIMITS ABOVE "A" LINE P.I. MORE THAN 7	
	TS SIBIE UNIC ENT	W _L < 50%	ΜL		INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY SANDS OF SLIGHT PLASTICITY	CLASSIFICATION		
200 SIEVE) SILTS BELOW "A" LINE NICHICAGE OFICANIC CONTENT	SIL BELOW :: NEGLIG ORGA	W _L > 50%	мн		INORGANIC SILTS, MICACEOUS OR DIATO- MACEOUS, FINE SANDY OR SILTY SOILS		5 BASED UPON LASTICITY CHART (see below)	
S.	F ON ARI	W _L < 30%	Cl		INORGANIC CLAYS OF LOW PLASTICITY, GRAVELLY, SANDY, OR SILTY CLAYS, LEAN CLAYS			
FINE-GRAINED SOILS HALF BY WEIGHT PASSI CLAYS ABOVE "A" LINFON PLASTICITY CHART NELSCHIELT CHART CONTENT		30 % < W _L < 50 %	CI .		INORGANIC CLAYS OF MEDIUM PLASTI- CITY, SILTY CLAYS			
CARCANIC ORGANIC ORGANIC CLAYS BELOW "A" LINE ON CHART	W _L > 50%	СН		INORGANIC CLAYS OF HIGH PLASTICITY, FAT CLAYS				
	W _L < 50%	ór		ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY	CONTENT H	THE NATURE OF THE FINE AS NOT BEEN DETERMINED, ATED BY THE LETTER "F", E.G.		
	ORGA SILTS CLA CLA BELOW 2	W _L > 50%	он		ORGANIC CLAYS OF HIGH PLASTICITY	SF IS A MIXT	URE OF SAND WITH SILT OR	
HIGHLY ORGANIC SOILS		Pi -		PEAT AND OTHER HIGHLY ORGANIC SOILS	STRONG CO	LOR OR ODOR, AND OFTEN TURE		

SPECIAL SYMBOLS



BEDROCK (Undifferentiated)



VOLCANIC ASH

c	٨ı		~	OM	D	٨ı	u	-	м	+	•
Э	v	_	v	.	_	v	N.	c.	N		3

oole down onemio						
FRACTION	U S STANDARD SIEVE SIZE	DEFINING RANGES OF PERCENTAGE BY WEIGHT OF MINOR COMPONENTS				
	PASSING RETAINED	PERCENT	DESCRIPTOR			
GRAVEL						
coarse fine	75 mm 19 mm 19 mm 4.75 mm	50 - 35	and			
SAND		35 - 20	some			
coarse	4.75mm 2.00mm					
m e dium	2.00 mm 4254 m	20 - 10	little			
fine	4254 m 754 m	10				
SILT (non plastic) or CLAY (plastic)	75 4 m	10 - 1	trace			

OVERSIZE MATERIAL

Rounded or eubrounded
COBBLES 75 mm to 200 mm
BOULDERS > 200 mm

Not rounded

ROCK FRAGMENTS > 75 mm

ROCKS > 0.76 cubic metre in volume

50-СН PLASTICITY CHART FOR SOILS PASSING 4254 m SIEVE <u>%</u> PLASTICITY INDEX CI 20 Cι CL - ML 20 40 70 80 LIQUID LIMIT (%)

- 1. ALL SIEVE SIZES MENTIONED ON THIS CHART ARE U.S. STANDARD, A.S.T.M. E.11.
- BOUNDARY CLASSIFICATIONS POSSESSING CHARACTERISTICS OF TWO GROUPS ARE GIVEN COMBINED GROUP SYMBOLS, E.G., GW-GC IS A WELL GRADED GRAVEL SAND MIXTURE WITH CLAY BINDER BETWEEN 5% AND 12%.

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